

APPENDIX B

HYDRAULICS REPORT

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B.1.0 BACKGROUND

A hydraulic analysis was performed in order to identify drainage structures with capacity issues. The hydraulic efficiency of the structures in the El Paso area was analyzed as follows:

- Normal depth calculations were performed along all study reaches to estimate channel capacity.
- CulvertMaster calculations were performed at channel crossings to estimate crossing capacity.
- Previous Federal Emergency Management Agency (FEMA) Hydrologic Engineering Center-River Analysis System (HEC-RAS) models listed below were reviewed to identify potential capacity issues.
 - Northeast Channel No. 2 (FEMA, 2004)
- Conduits exhibiting poor performance were identified as part of the prioritization exercise performed in Task 2 of the Stormwater Master Plan (SMP), and capacities were estimated.
- Pump stations exhibiting poor performance were identified as part of the prioritization exercise performed in Task 2 of the SMP. The majority of the pump station analysis was conducted during the Alternatives Analysis phase, and is discussed further in Appendix D.

This appendix will present the basic methodologies associated with the calculations performed as part of the hydraulic evaluation process. An overview of the project area is provided on Figure B-1.

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B.2.0 DATA SOURCES

Table B-1 lists the sources used in the hydraulic analysis, as well as the specific calculation(s) each source was used for.

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B.3.0 CHANNEL ANALYSES

B.3.1 Method Overview

As part of the hydraulic study, channel capacities were analyzed for cross-sections located along each study channel using Manning's Normal Depth assumption. Channel geometry was estimated from a variety of sources including the previous Work Order 1 Study (URS Corporation [URS], 2007), the El Paso Water Utilities (EPWU) drainage layer, Texas Department of Transportation (TxDOT) contour data (TxDOT, 2004), City of El Paso Orthophotos (City of El Paso, 2006), existing HEC-RAS models (FEMA, 2004), and site visits. Generally, the Work Order 1 geometry was the primary data source utilized unless there was an obvious conflict with aerial imagery or contours. In these instances, the additional data sources listed above were used to verify accurate geometry. Cross-sections were analyzed along channels at locations of varying geometry or channel slope to estimate the capacity for the entire study channel. Capacity estimates were performed using Bentley FlowMaster, or an equivalent Normal Depth Method.

ArcView shapefiles were digitized to show the approximate cross-section locations corresponding to the capacities estimated for each of the regions studied. For each cross-section analyzed, the nearest downstream Hydrologic Engineering Center-Hydraulic Modeling System (HEC-HMS) flow node was identified. Each node has an associated 10, 25, 50, 100, and 500-year storm flow. Using functions created in Excel, the estimated cross-section capacity was compared with the downstream node's storm flows to interpolate an approximate return period corresponding to the channel capacity. In some instances, the nearest downstream flow node contained a large contributing watershed area that did not enter the channel until downstream of the cross-section being analyzed. In such cases, the downstream flows were sometimes scaled down using an area reduction factor equal to the area of the watershed contributing upstream of the cross-section, divided by the total watershed area contributing to the HMS flow node. This step was typically performed during the conceptual design phase where deemed appropriate in order to more accurately size proposed drainage structures.

Channels that were found to be potentially undersized to convey the 100-year storm event were classified as either Case I or Case II. Channels were denoted as Case I if floodwaters would be contained within the channel overbanks. Channels were denoted as Case II if floodwaters had the potential to exit the channel overbanks and enter adjacent watersheds. These classifications were used during the prioritization of projects, discussed in Appendix E. Figure B-2 illustrates Case I and Case II flooding scenarios.

B.3.2 Channel Analysis - Central Region

Channel flow capacities and associated return periods were calculated for the Central Region as described above in Section B.3.1. Plan sets for the Government Hills Channel were provided by EPWU (Parkhill, Smith, & Cooper, Inc. [PSC], January 2007a), to supplement Work Order 1 geometry. Site visits were also performed to determine channel geometries that were not available in Work Order 1.

Conduit flow capacities and associated return periods were also calculated for major large conduits. Conduits that have a diameter larger than 42 inches and are not the primary drainage structures were considered in the analysis. These conduits do not serve as drainage structures for streets, but rather as major storm water corridors from reservoirs to outlets. A hydraulic conduit analysis program called FlowMaster was used to estimate the return intervals. Results for the Central Region channel analysis are provided in Table B-2 and Figures B-3 through B-5 located at the end of this Appendix.

B.3.3 Channel Analysis - East Side Region

Channel flow capacities and associated return periods were calculated for the East Side Region as described in Section B.3.1. Additionally, FlowMaster was used to calculate the normal depth for each channel. Additional data was obtained from record drawings for channel data that was not available in Work Order 1. Results are provided in Table B-3 and Figures B-6 through B-11 located at the end of this Appendix.

B.3.4 Channel Analysis - Mission Valley Region

Channel flow capacities and associated return periods were calculated for the Playa and Mesa Drains in Mission Valley as described above. A bed profile drawing was created for both drains and the channel capacity was calculated at any location where there was a significant change in slope. Due to discrepancies in geometry among the various sources for many of the cross sections in Mission Valley, a site visit was necessary to obtain more precise measurements. Results are provided in Table B-4 and Figures B-12 through B-14 located at the end of this Appendix.

B.3.5 Channel Analysis - Northeast Region

Channel flow capacities and associated return periods were calculated for the Northeast Region as described above. Additional data was required at Electric Ditch Diversion Channel due to discrepancies in geometry among the various sources, so a site visit was performed to obtain more precise measurements. Additionally, the 2004 FEMA Flood Hazard HEC-RAS Model (FEMA, 2004) was used to approximate capacities along the Northeast Channel No. 2, utilizing the hydrologic flows from this study. Results for the Northeast Region channel analysis are provided in Table B-5 and Figures B-15 through B-17 located at the end of this Appendix.

B.3.6 Channel Analysis - Northwest Region

Channel flow capacities and associated return periods were calculated for the Northwest Region as described above. Due to the new development of the area around Flow Path No. 38, additional data was required. Data was gathered from the "West Hills Unit Twenty Three Off Site Drainage" as-builts and a site visit was performed to confirm the finding. Results are provided in Table B-6 and Figures B-18 through B-24 located at the end of this Appendix.

B.3.7 Channel Analysis - West Central Region

Flow capacities and associated return periods for the channels in the West Central Region were calculated as described above. Additional data was required for the beginning of Flow Path No. 20 due to inconsistencies in the geometry and flow path among the various data sources. A site visit was performed to confirm the flow path and obtain more precise measurements of the channel. Results are provided in Table B-7 and Figure B-25 located at the end of this Appendix.

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B.4.0 CROSSING STRUCTURE ANALYSES

B.4.1 Method Overview

Initially, culvert capacities obtained from Work Order 1 (URS, 2007) were utilized for this analysis where available. However, culverts analyzed in Work Order 1 were modeled in CulvertMaster with the assumption that there was no tailwater present. In order to ascertain the effect of tailwater on the culverts included in this study, a downstream channel cross section was entered for each culvert into CulvertMaster and new capacities were estimated. Bridges were also analyzed in CulvertMaster. The area under the bridge was estimated and a culvert of equal cross-sectional flow area, matching the height and width of the bridge as much as possible, was entered into CulvertMaster. Crossing structure information was obtained from a variety of sources, including the Work Order 1 Study, the EPWU drainage layer (EPWU, 2008), as-builts, existing HEC-RAS Models, TxDOT topography (TxDOT, 2004), orthophotography (City of El Paso, 2006), and site visits.

CulvertMaster uses several parameters, including the upstream invert elevation, the downstream invert elevation, and slope to analyze a culvert. In some instances, survey data was available with the appropriate invert elevations upstream and downstream of the culverts as well as top of road elevations. Other culverts were missing this information, but did have inverts that were included as attributes to the “Conduit_Join_Selection.shp” or “Culverts_Selection.shp” shapefiles, which were originally provided by the City of El Paso. However, the inverts included in these shapefiles were based on a different vertical datum than the survey used. The datum adjustment for the City of El Paso inverts was approximately +10 feet when compared to the survey points. If information on the culvert inverts was not available, the slope of the culvert was assumed to match that of the channel.

As mentioned above, a parameter that is used by CulvertMaster to calculate tailwater depth is the channel geometry downstream of the culvert. This geometry was approximated based on the TxDOT topography and a representative cross-section was entered into CulvertMaster. The TxDOT topography vertical datum was found to be consistent with the survey utilized, but not the City of El Paso shapefiles. Therefore, in each case where inverts were not available from survey points, but were available in the City of El Paso shapefiles, the elevations from the downstream channel cross section were uniformly adjusted in CulvertMaster so that the bottom elevation matched the downstream invert elevation from the City of El Paso shapefiles.

As with the channel analysis described previously, ArcView shapefiles were digitized to show the approximate crossing locations. For each crossing analyzed, the nearest downstream HEC-HMS flow node was identified. Each node has an associated 10-, 25-, 50-, 100-, and 500-year design flow. Using functions created in Excel, the estimated cross-section capacity was compared with the downstream node’s storm flows to interpolate an approximate return period corresponding to the channel capacity.

In some instances, the nearest downstream flow node contained a large contributing watershed area that did not enter the channel until downstream of the cross-section being analyzed. In such cases, the downstream flows were sometimes modified using an area reduction factor equal to the area of the watershed contributing to upstream of the crossing divided by the total watershed area contributing to the HMS flow node. This step was typically performed during the conceptual design phase where deemed appropriate in order to more accurately size proposed drainage structures.

B.4.2 Crossing Structure Analysis - Central Region

Crossing capacities and associated return periods were calculated for the Central Region as described above. Results are provided in Table B-8 and Figures B-3 through B-5 located at the end of this Appendix.

B.4.3 Crossing Structure Analysis - East Side Region

Crossing capacities and associated return periods were calculated for the East Side Region as described in Section B.4.1. Tailwater information was assumed to be the soffit elevation at the downstream end of the hydraulic structure. Headwater information was obtained from record drawings. If no record drawings were available, headwater was assumed to be the upstream soffit elevation plus one foot. Results are provided in Table B-9a and Figures B-6 through B-11 located at the end of this Appendix.

Culvert crossings along the IH-10 corridor between Robert E. Lee Road and eastern City limits were analyzed slightly differently than described above using CulvertMaster to identify any potential problems along the IH-10 corridor. Headwater elevation for culvert analysis was identified per record drawings or assumed to be the upstream soffit elevation plus one foot. Tailwater elevation for culvert analysis was assumed as the downstream soffit elevation. The culvert crossings were identified by TxDOT station number at the centerline of the structure and the centerline of the roadway with the use of record drawing information. Flows for each crossing were assumed as the complete watershed flow equally divided by the number of crossings in the watershed. This type of analysis is cursory in nature and further analysis on crossings identified as not "At Capacity" must be performed. The results of the analysis of the IH-10 crossings is provided in Table B-9b.

B.4.4 Crossing Structure Analysis - Mission Valley Region

Only crossings on the Playa and Mesa Drains were analyzed for the Mission Valley Region. These crossing capacities were calculated as described above. The crossing capacities were not associated with a specific return interval, but rather a percentage of the upstream channel capacity that could be passed. This method was more practical for the Mission Valley Region because the Playa and Mesa drains are both undersized. Results are provided in Table B-10 and Figures B-12 through B-14 located at the end of this Appendix.

B.4.5 Crossing Structure Analysis - Northeast Region

Crossing capacities and associated return periods were calculated for the Northeast Region as described above. Results are provided in Table B-11 and Figures B-15 through B-17 located at the end of this Appendix.

B.4.6 Crossing Structure Analysis - Northwest Region

Crossing capacities and associated return periods were calculated for the Northwest Region as described above. Results are provided in Table B-12 and Figures B-18 through B-24 located at the end of this Appendix.

B.4.7 Crossing Structure Analysis - West Central Region

Crossing capacities and associated return periods were calculated for the West Central Region as described above. Results are provided in Table B-13 and Figure B-25 located at the end of this Appendix.

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B.5.0 CONDUITS ANALYSES - CENTRAL REGION ONLY

In addition to the channel and crossing analysis performed for the other regions, several significant conduits were analyzed in the Central Region. Conduit capacities were obtained from Work Order 1 where available. Work Order 1 used both CulvertMaster and FlowMaster to analyze conduits. Each conduit larger than 42 inches was re-analyzed along with any large conduits that were not previously analyzed. Information was gathered from as-built plan sheets provided by EPWU, TxDOT Plans, Work Order 1 data, and site visits.

FlowMaster uses several parameters, including the upstream invert elevation, the downstream invert elevation, and slope to analyze a conduit. In some instances, survey data was available with the appropriate invert elevations upstream and downstream of the conduits. Some conduits were missing this information, but did have inverts that were included as attributes to the "Culverts_Selection.shp" shapefile, which were originally provided by the City of El Paso. However, the inverts included in these shapefiles were based on a different vertical datum than the survey used. The datum adjustment to the City of El Paso inverts was approximately +10 feet when compared to the survey points.

For each conduit analyzed, the nearest upstream HEC-HMS flow node was identified. Each node has an associated 10-, 25-, 50-, 100-, and 500-year storm flow. Using functions created in Excel, the estimated conduit capacity was compared to the peak discharge of the selected node. The maximum capacity of the conduit was then correlated with a specific return period, interpolated between bounding storm events. The result is a return period associated with the conduit that can be used in design. Conduits that could not pass the 100-year storm flows were deemed undersized and further analysis was required.

Conduit capacities and associated return periods were calculated for the Central Region as described above. Results are provided in Table B-14 and Figures B-3 through B-5 located at the end of this Appendix B.

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TABLES

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Table B-1. Data Sources Used in Hydraulic Analysis

| Source | Used For |
|--|---------------------------------------|
| Bentley, CulvertMaster. | Channel Analysis Conduit Analysis |
| Bentley, FlowMaster. | Crossing Analysis Channel Analysis |
| City of El Paso, 2006. Orthophotography. | Crossing Analysis Channel Analysis |
| El Paso Storm Water Utility, 2008. El Paso Drainage Layer (incomplete). | Crossing Analysis Channel Analysis |
| Mapping Alliance Partnership 6 (MAP 6) - FEMA, 2004. Flood Hazard Study. | Channel Analysis |
| Moreno Cardenas Inc. (MCi), February 2008. Drainage Study and Report (Existing Conditions) for Interstate Highway 10. | Crossing Analysis |
| Parkhill, Smith, & Cooper, Inc. (PSC), January 2007a. Government Hill Outfall Durazno Neighborhood. Storm 2006 Drainage Repairs Project. | Channel Analysis |
| Texas Department of Transportation (TxDOT), El Paso Office, 2004. Topography | Crossing Analysis Channel Analysis |
| URS Corporation (URS), 2007. Electronic Data-Appendix A - Tabulated Results, Table A-2 Hydraulics and Assessment Summary, Work Order 1. | Crossing Analysis Channel Analysis |
| URS, 2007. Electronic Data-Appendix I, Drainage System Evaluation and Audit Report, Drainage On-Call Services, Work Order 1 | Crossing Analysis Channel Analysis |
| URS, 2008. Electronic Data - Appendix E, Dam Analysis Report, Drainage On-Call Services, Task 3 of Work Order 3. | Channel Analysis |

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Table B-2. Channel Capacity Summary - Central Region

| CENTRAL REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|---------------------------|-------------------|------------|-------------|------------------------------|--|--------------------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| El Paso Rock Quarries | Irregular Geometry - 4pts | | | 0.035 | 7000 | Davis-Seaman Rd to Alabama | R_El_Paso_Rock_Quarries | 50 |
| El Paso Rock Quarries | 15 | 2:1 | 7.9 | 0.045 | 4000 | Near Alabama St | R_El_Paso_Rock_Quarries | 50 |
| Government Hills Channel | 15 | 2:1 | 4 | 0.013 | 1800 | Coldwell Elementary School | J_Gov Hills North Inflow | 100 |
| Government Hills Channel | Irregular Geometry - 6pts | | | 0.013 | 1900 | West of Boone Street | J_Gov Hills North Inflow | 100 |
| Government Hills Channel | 10 | 2:1 | 4 | 0.013 | 1400 | Altura Rd | J_Gov Hills North Inflow | 100 |
| Government Hills Channel | 10 | 2:1 | 4 | 0.013 | 1400 | US of Cumberland Dr | J_Gov Hills North Inflow | 100 |
| Government Hills Channel | 10 | 2:1 | 4 | 0.013 | 1400 | Between Chester and Oxford Ave. | J_Gov Hills North Inflow | 100 |
| Government Hills Channel | 15 | 2:1 | 4 | 0.013 | 1700 | Upstream of Hueco Ave. | J_Gov Hills North Inflow | 100 |
| Mckelligon Channel | Irregular Geometry - 6pts | | | 0.04 | 6500 | At Mckelligon Canyon Road | J_Mckelligon_Outflow | 100 |
| Mckelligon Channel | Irregular Geometry - 6pts | | | 0.04 | 7900 | Between Davis Seamon & Mckelligon | J_Mckelligon_Outflow | 100 |
| Pershing Discharge Channel | 10 | .65:0 | 15 | 0.012 | 8200 | Pershing Outfall Channel | J_Gov Hills North Inflow | 100 |
| Pershing Discharge Channel | 25 | .5:1 | 8.75 | 0.016 | 8800 | Pershing Outfall Near Gov. Hills Channel | J_Gov Hills North Inflow | 100 |
| Pollard Ditch | 8 | 1:1 | 5 | 0.02 | 1400 | Porter to Embankment | A_Pollard Ditch | 25 |
| Van Buren Discharge Channel | Irregular Geometry - 8pts | | | 0.04 | 3100 | West of Oklahoma Street | J_Van Buren Ditch Inflow | 100 |
| Van Buren Discharge Channel | 15 | 1:1 | 5 | 0.035 | 2200 | Parallel to Oklahoma St | J_Van Buren Ditch Inflow | 100 |

Table B-2. Channel Capacity Summary - Central Region (Continued)

| CENTRAL REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|---------------------------|-------------------|------------|-------------|------------------------------|-------------------------------|--------------------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Van Buren Discharge Channel | Irregular Geometry - 6pts | | | 0.038 | 5100 | East of Alabama | J_Van Buren Ditch Inflow | 100 |
| Copia_Ditch | Irregular Geometry - 4pts | | | 0.013 | 5200 | Downstream of Savannah Street | J_Copia_Ditch_US | 100 |
| Copia_Ditch | Irregular Geometry - 4pts | | | 0.013 | 3200 | Downstream of Altura Avenue | J_Copia_Ditch_US | 100 |
| Copia_Ditch | Irregular Geometry - 4pts | | | 0.013 | 3600 | Downstream of Richmond Avenue | J_Copia_Ditch_US | 100 |
| Copia_Ditch | Irregular Geometry - 4pts | | | 0.013 | 1600 | Downstream of Louisville Ave | J_Copia_Ditch_US | 100 |
| Copia_Ditch | Irregular Geometry - 4pts | | | 0.013 | 2700 | Downstream of Lebanon Ave | J_Copia_Ditch_US | 100 |
| Copia_Ditch | Irregular Geometry - 4pts | | | 0.035 | 300 | 50' NE of RR tracks | J_Copia_Ditch_US | 100 |
| Copia_Ditch | Irregular Geometry - 4pts | | | 0.011 | 3100 | Below RR tracks | J_Copia_Ditch_US | 100 |

Table B-3. Channel Capacity Summary - East Side Region

| EAST SIDE REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|-------------------|-------------|------------|-------------|------------------------------|---|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Bluff Channel Cross Section 1 | 10 | 1.5 | 4 | 0.013 | 1442.34 | Beginning of Channel to Esther Lama Drive | 9J | 40 |
| Bluff Channel Cross Section 2 | 12 | 1.33 | 6 | 0.013 | 3524.49 | Esther Lama Drive to Drop at Old Basin | 9J | >100 |
| Bluff Channel Cross Section 3 | 35 | 2 | 9 | 0.013 | 6987.34 | Drop at Old Basin to IH-10 | 9J | >100 |
| Fort Bliss Spur Drain Cross Section 1 | 4 | 1.5 | 4 | 0.013 | 1014 | Airway Boulevard to Robert E. Lee Boulevard | A-PD-1 | >100 |
| Fort Bliss Spur Drain Cross Section 2 | 4 | 1.5 | 4.5 | 0.013 | 964 | Edgemere Boulevard to Railroad Crossing | A-PD-1 | >100 |
| Fort Bliss Spur Drain Cross Section 3 | 4 | 1.5 | 4 | 0.013 | 907 | Railroad Crossing to Half-way IH-10 | A-PD-1 | >100 |
| Fort Bliss Spur Drain Cross Section 4 | 4 | 1.5 | 3 | 0.013 | 1204 | Half-way IH-10 to Upstream Culvert Crossing | A-PD-1 | >100 |
| Fort Bliss Spur Drain Cross Section 5 | 4 | 1.5 | 4.5 | 0.013 | 1460 | Upstream Culvert Crossing to IH-10 | A-PD-1 | >100 |
| Sunmount Channel | 10 | 5 | 7 | 0.030 | 2720 | W.H. Burges Drive to Viscount Boulevard | A_Sunmount | >100 |
| Vanderbilt Channel | 10 | 1.5 | 5 | 0.013 | 1817 | Beginning of Channel to Jesuit Basin | A-LL-8 | >100 |
| RV Channel Cross Section 1 | 50 | 0 | 5.17 | 0.030 | 5184 | Upstream Culvert Crossing at Pine Springs Drive | 124A & 124E | >100 |
| RV Channel Cross Section 2 | 20 | 1 | 4 | 0.013 | 3580 | Pine Springs Drive to Rojas Drive | 124JW | >100 |

Table B-3. Channel Capacity Summary - East Side Region (Continued)

| EAST SIDE REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|-------------------|-------------|------------|-------------|------------------------------|---|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| RV Channel Cross Section 3 | 50 | 0 | 2.83 | 0.030 | 1324 | Rojas Drive to RV Drive | 124JW | >100 |
| RV Channel Cross Section 4 | 30 | 2 | 4 | 0.013 | 3184 | RV Drive to RV/Mercantile Channel Junction | 124JW | >100 |
| Mercantile Channel Cross Section 1 | 45.67 | 0 | 5 | 0.030 | 2049 | Upstream Culvert Crossing at Mercantile Avenue | 124C | >100 |
| Mercantile Channel Cross Section 2 | 20 | 1 | 5 | 0.013 | 3998 | Mercantile Avenue to Rojas Drive | 124JE | >100 |
| Mercantile Channel Cross Section 3 | 30 | 2 | 4.5 | 0.013 | 3033 | Rojas Drive to RV/Mercantile Channel Junction | 124JE | >100 |
| Joe Battle Channel Cross Section 1 | 8 | 2 | 6 | 0.013 | 1212 | Beginning of Channel to Channel Turn | A-AM-5 | >100 |
| Joe Battle Channel Cross Section 2 | 10 | 2 | 6 | 0.013 | 3689 | Channel Turn to Pond | A-AM-5 | >100 |
| Peter Hurd Channel | 6 | 1 | 3.5 | 0.013 | 622 | Beginning of Channel to Vista del Sol Drive | A-AM-7 | >100 |
| Lorne Channel Cross Section 1 | 8 | 1 | 2.5 | 0.013 | 307 | Limerick Road to Lorne Road | A-PD-6 | 25 |
| Lorne Channel Cross Section 2 | 3 | 1 | 3 | 0.013 | 196 | Lorne Road to Pond | A-PD-6 | 5 |

Table B-4. Channel Capacity Summary - Mission Valley Region

| MISSION VALLEY REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|--|-------------------|-------------------|------------|-------------|------------------------------|----------------------------|--|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Mesa Drain | 15 | 2:1 | 9 | 0.03 | 750 | Northloop | J_Mesa Drain w Phelps Dodge | <10 |
| Mesa Drain | 15 | 2:1 | 9 | 0.03 | 1375 | Just East of Mauer | J_Mesa Drain w Lafayette Drw | 10 |
| Mesa Drain | 15 | 1.5:1 | 10 | 0.03 | 1200 | Just East of Lomaland | J_Mesa Drain w Lomaland | 10 |
| Playa Drain | 12 | 2:1 | 7 | 0.013 | 645 | Little Flower | J_Playa Drain w Basin A | 10 |
| Playa Drain | 24 | 2:1 | 8 | 0.03 | 1275 | End of Conduit U/S Knights | R_Playa Drain Conduit and A_Playa Drain B (Area Reduction) | 65 |
| Playa Drain | 20 | 1.5:1 | 8 | 0.03 | 1000 | U/S of Yarbrough | R_Playa Drain Conduit and A_Playa Drain B (Area Reduction) | 50 |
| Playa Drain | 15 | 1.5:1 | 7 | 0.03 | 375 | Whittier | R_Playa Drain Conduit and A_Playa Drain B (Area Reduction) | <10 |

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Table B-5. Channel Capacity Summary - Northeast Region

| NORTHEAST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|--------------------|------------|-------------|------------------------------|-----------------------------------|-----------------------------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Bossworth | 14 | 1 | 10 | 0.016 | 13000 | Morningside Cir to Byron | A_Bossworth U/S | >100 |
| Bossworth | 10 | 2 | 5 | 0.035 | 1500 | Byron to Dyer | J_Bossworth Ch D/S | >100 |
| Bossworth | 25 | 2 | 5 | 0.035 | 3300 | Byron to Dyer - Section 2 | J_Bossworth Ch D/S | >100 |
| Corps of Engineers | 14 | 1 | 30 | 0.035 | 65000 | Edgar Park Ave - Mountain View Rd | J_Ft Bliss Div Ch U/S | >100 |
| Diana Ditch | 28 | Irregular Geometry | 4.8 | 0.016 | 814 | X-Section 1 | J_Diana Ditch D/S | 10 |
| Diana Ditch | 9 | Irregular Geometry | 4.5 | 0.016 | 513 | X-Section 2 | J_Diana Ditch U/S | 10 |
| Electric Ditch Div | 75 | 1 | 2 | 0.03 | 755 | U/S McCombs East | J_Electric Ditch (Area Reduction) | 100 |
| Electric Ditch Div | 50 | 1 | 4 | 0.03 | 1600 | McCombs East | J_Electric Ditch | >100 |
| Electric Ditch Div | 30 | Irregular Geometry | 6.5 | 0.03 | 1200 | D/S McCombs East | J_Electric Ditch | >100 |
| Fort Bliss Div | 15 | 2 | 10 | 0.04 | 6900 | US to Dyer St | J_Ft Bliss Div Ch Dyer St | >100 |
| Fusselman Out | Irregular Geometry | | | 0.035 | 5000 | | S_Fusselman Dam | >100 |
| Greenbelt Levee System | 1000 | 4 | 5 | 0.04 | 46000 | US 54 | J_Green Belt Levee D/S | >100 |
| Hondo Pass | 60" Conduit | N/A | N/A | 0.016 | 322 | Mercury Street | A_Hondo Pass Ch | >100 |
| Northgate Div | 0 | 2 | 15 | 0.04 | 7900 | US end to Ft Bliss | A_Northgate Div Ch | >100 |
| Northgate Interceptor Ch | 30 | 2/3 | 8 | 0.03 | 2400 | | A_Northgate Int Ch | >100 |
| Northgate Outlet | 10 | 0 | 3.67 | 0.016 | 491 | W of Gateway S to GWS | J_Northgate Dam Out | >100 |
| Northgate Outlet | 15 | 1 | 10 | 0.035 | 3000 | E of GWS to Ponding | J_Northgate Dam Out | >100 |

Table B-5. Channel Capacity Summary - Northeast Region (Continued)

| NORTHEAST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|------------------------|-----------------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| PSB No 1 | 23 | 1 | 3 | 0.016 | 895 | U/S end to Gateway S | J_PSB Ch1 Jct1 | >100 |
| PSB No 1 | 51 | 1 | 12.5 | 0.013 | 30000 | Kenworthy Crossing | J_PSB Ch1 Jct2 | >100 |
| PSB No 1 | 57 | 1 | 13.5 | 0.013 | 37000 | Capt Valtr St Crossing | J_PSB Ch1 Jct2 | >100 |
| PSB No 1 | 30 | 1 | 15 | 0.016 | 36000 | Gateway S to McComb | J_PSB Ch1 Jct2 | >100 |
| PSB No 1 | 45 | 1 | 4 | 0.016 | 3600 | | J_PSB Ch 1 D/S | >100 |
| PSB No 2 | 30 | 0 | 3 | 0.03 | 469 | US of Gateway | J_PSB Ch 2 at Gateway | 30 |
| PSB No 2 | 15 | 1 | 12 | 0.02 | 4800 | DS of Gateway | J_PSB Ch 2 at Rushing | >100 |
| PSB No 2 | 10 | 1 | 15 | 0.03 | 3500 | D/S of Sun Valley | J_PSB Ch 2 at Rushing | >100 |
| PSB No 2 | 13 | 1.5 | 6.5 | 0.03 | 2400 | D/S of Kentworthy | J_PSB Ch 2 at Rushing | >100 |
| PSB No 2 | 26 | 1.5 | 6 | 0.03 | 2700 | D/S of McCombs | J_PSB Ch 2 D/S | >100 |
| PSB No 2 | 20 | 1 | 10 | 0.035 | 2500 | Pambino Road | J_PSB Ch 2 D/S | >100 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 100 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 80 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 10 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 20 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 100 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 10 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 100 |

Table B-5. Channel Capacity Summary - Northeast Region (Continued)

| NORTHEAST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|--------------------|------------|-------------|------------------------------|----------------------------|--|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 40 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 90 |
| PSB No 2 | Irregular Geometry | | | 0.035 | N/A | FEMA HEC-RAS Model | J_PSB Ch 2 D/S | 100 |
| Railroad Drive | 8 | 3 | 8 | 0.03 | 1700 | U/S of Threadgill | J_RR Drain U/S of Tobin (Area Reduction) | >100 |
| Railroad Drive | 50 | 1.5 | 6 | 0.03 | 2800 | Threadgill to Statler | J_RR Dr U/S Statler Ch | >100 |
| Railroad Drive | 65 | 1.5 | 6 | 0.03 | 3600 | Statler to Fort Bliss Sump | J_RR Ditch Downstream | >100 |
| Range Dam Outlet | ~18 | Irregular Geometry | ~4 | 0.016 | 600 | Section 1 | J_Army Ditch | >100 |
| Range Dam Outlet | ~12 | Irregular Geometry | ~5 | 0.016 | 800 | Section 2 | J_Army Ditch | >100 |
| Range Dam Outlet | ~33 | Irregular Geometry | ~7 | 0.016 | 1900 | Section 3 | J_Army Ditch | >100 |
| Statler Ditch | 11 | Irregular Geometry | ~4 | 0.013 | 136 | X-Section 1 | A_Statler Ditch | 20 |
| Sunrise | 10 | 1 | 4 | 0.016 | 1100 | Dyer to Diana | J_Sunrise Ch D/S | >100 |
| Threadgill | ~26 | Irregular Geometry | ~10 | 0.03 | 3000 | Section 1 | J_Tobin Drain D/S | >100 |
| Threadgill | ~21 | Irregular Geometry | ~4 | 0.016 | 865 | Section 2 | J_Tobin Drain D/S | 20 |
| Threadgill | ~10 | Irregular Geometry | ~4 | 0.016 | 590 | Section 3 | J_Tobin Drain U/S Army Ditch | 20 |
| War Road | 28 | 1 | 6 | 0.016 | 4800 | DS end to Loma Rd | J_War Road Channel | >100 |
| Western Fwy | 43 | 1.5 | 6 | 0.035 | 2500 | McComb Rd | J_W Fwy D/S | >100 |
| Western Fwy | 32 | 1 | 10 | 0.035 | 4500 | DS end | J_W Fwy D/S | >100 |

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Table B-6. Channel Capacity Summary - Northwest Region

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Arroyo 1A | Irregular Geometry | | | 0.035 | >1291 | US of confluence with High Ridge Channel | A1A | >100 |
| Arroyo 4 | Irregular Geometry | | | 0.016 | >1141 | DS of IH-10 | A4_1 | >100 |
| Arroyo 4 | Irregular Geometry | | | 0.035 | >1141 | Gem St. to IH-10 | A4_1 | >100 |
| Arroyo 4 | Irregular Geometry | | | 0.03 | >1141 | Desert Trail Rd. to Gem St. | A4_1 | >100 |
| Arroyo 4 | Irregular Geometry | | | 0.039 | >1141 | Mesa St. to Desert Trail Rd. | A4_1 | >100 |
| Arroyo 4 | Irregular Geometry | | | 0.04 | >1141 | Resler Dr. to Mesa St. | A4_1 | >100 |
| Arroyo 4 | Irregular Geometry | | | 0.035 | >1141 | Northwind Dr. to El Puente St. | A4_1 | >100 |
| Arroyo 4 | Irregular Geometry | | | 0.035 | >1141 | Broadmoor Dr to Northwind Dr. | A4_1 | >100 |
| Arroyo 5 | Irregular Geometry | | | 0.03 | >1519 | N. Resler Dr to Keystone Dam | A5_1 | >100 |
| Arroyo 5 | Irregular Geometry | | | 0.03 | >1519 | 1500 ft DS of Mesa St. to N. Resler Dr. | A5_1 | >100 |
| Arroyo 5 | Irregular Geometry | | | 0.03 | >1519 | Mesa St. to 1500 ft DS of Mesa St. | A5_1 | >100 |
| Doniphan Ditch | 10 | 1.7:1 | 3 | 0.03 | 160 | Dona Ana County Rd. to outlet | DD_1 | 5 |
| Doniphan Ditch | 19 | 3:1 | 4.5 | 0.03 | 685 | Frontera Rd. to Dona Ana County Rd. | DD_1 | 90 |
| Doniphan Ditch | 20 | 3:1 & 4:1 | 3.5 | 0.03 | 320 | Bird Ave. to Frontera Rd. | DD_1 | 40 |
| Doniphan Ditch | 30 | 3.3:1 | 3 | 0.03 | >242 | Sunset Dr to Bird Ave. | DD_1 | >100 |
| Doniphan Ditch | Irregular Geometry | | | 0.03 | 27 | DS of White Spur Drain to Sunset Dr. | DD_1 | 25 |
| Doniphan Ditch | Irregular Geometry | | | 0.03 | 27 | US of White Spur Drain | DD_3 | 2 |

Table B-6. Channel Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 38 | Irregular Geometry | | | 0.033 | >896 | 300 ft US XS01 to confluence with Resler Channel | FPN38_1 | >100 |
| Flow Path No. 38 | Irregular Geometry | | | 0.032 | >896 | 100 ft US XS02 to 300 ft US XS01 | FPN38_1 | >100 |
| Flow Path No. 38 | Irregular Geometry | | | 0.035 | >896 | Desert Blvd. to 100 ft US XS02 | FPN38_1 | >100 |
| Flow Path No. 38 | Irregular Geometry | | | 0.035 | >896 | Northwestern Dr. to Desert Blvd. | FPN38_1 | >100 |
| Flow Path No. 38 | Irregular Geometry | | | 0.035 | >896 | 300 ft US XS05 to Northwestern Dr. | FPN38_1 | >100 |
| Flow Path No. 38 | Irregular Geometry | | | 0.03 | >896 | Confluence of FPN38A to 300 ft US XS05 | FPN38_1 | >100 |
| Flow Path No. 38 | 10 | 1:1 | 4 | 0.016 | >744 | Corona Del Sol to confluence of FPN38A | FPN38_3 | >100 |
| Flow Path No. 38 | Irregular Geometry | | | 0.025 | >744 | Redd Rd. to Corona Del Sol | FPN38_3 | >100 |
| Flow Path No. 38 | Irregular Geometry | | | 0.035 | >0.2 | US of Redd Rd. | FPN38_4 | >100 |
| Flow Path No. 38A | Irregular Geometry | | | 0.031 | >1392 | 1000 ft US XS 1.0 to confluence with FPN38 | FPN38A | >100 |
| Flow Path No. 38A | Irregular Geometry | | | 0.035 | >1392 | US of 1000 ft US XS 1.0 | FPN38A | >100 |
| Flow Path No. 38B | Irregular Geometry | | | 0.031 | >768 | 1000 ft US XS 2.0 to confluence with FPN38 | FPN38B | >100 |
| Flow Path No. 38B | Irregular Geometry | | | 0.04 | >768 | US of 1000 ft US XS 2.0 | FPN38B | >100 |
| Flow Path No. 39A | Irregular Geometry | | | 0.03 | >2509 | 300 ft US XS 1.0 to confluence with UN23 | FPN39A_1 | >100 |
| Flow Path No. 39A | Irregular Geometry | | | 0.035 | >2509 | IH-10 to 300 ft US XS 1.0 | FPN39A_1 | >100 |
| Flow Path No. 39A | Irregular Geometry | | | 0.031 | >2509 | FPN39A_R to IH-10 | FPN39A_1 | >100 |
| Flow Path No. 39A | Irregular Geometry | | | 0.033 | >2497.5 | Resler Dr. to FPN39A_R | FPN39A_2 | >100 |

Table B-6. Channel Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 39A | 30 | 39.33:1 & 13.17:1 | 3 | 0.035 | >2497.5 | US of Resler | FPN39A_2 | >100 |
| Flow Path No. 39A Redirect | 24 | 1.4:1 | 5.5 | 0.016 | >2675 | 475 ft DS of XS 2.0 to confluence with FPN40 | FPN40_2 | >100 |
| Flow Path No. 39A Redirect | Irregular Geometry | | | 0.034 | >2675 | Confluence with FPN39A to 330 ft US of XS 1.0 | FPN40_2 | >100 |
| Flow Path No. 40 | Irregular Geometry | | | 0.032 | 2000 | 150 ft US of XS 1.0 to confluence with Unnamed Stream No. 23 | FPN40_1 | 15 |
| Flow Path No. 40 | Irregular Geometry | | | 0.031 | >4361.9 | IH-10 to 150 ft US of XS 1.0 | FPN40_1 | >100 |
| Flow Path No. 40 | Irregular Geometry | | | 0.033 | >4361.9 | Transmountain to IH-10 | FPN40_1 | >100 |
| Flow Path No. 40 | Irregular Geometry | | | 0.03 | >1891.4 | US of Transmountain | FPN40_3 | >100 |
| Flow Path No. 42 | Irregular Geometry | | | 0.035 | >937 | 200 ft US of XS 1.0 to IH-10 | FPN42 | >100 |
| Flow Path No. 42 | Irregular Geometry | | | 0.033 | >937 | 600 ft DS of XS 3.0 to 20 ft US of XS 1.0 | FPN42 | >100 |
| Flow Path No. 42 | Irregular Geometry | | | 0.034 | >937 | 600 ft DS of XS 4.0 to 600 ft DS of XS 3.0 | FPN42 | >100 |
| Flow Path No. 42 | Irregular Geometry | | | 0.033 | >937 | 600 ft DS of XS 5.0 to 600 ft DS of XS 4.0 | FPN42 | >100 |
| Flow Path No. 42 | Irregular Geometry | | | 0.033 | >937 | US of 600 ft DS of XS 5.0 | FPN42 | >100 |
| Flow Path No. 42 Trib 1 | Irregular Geometry | | | 0.035 | >851 | Confluence with Unknown 1 to IH-10 | FPN42T1_1 | >100 |
| Flow Path No. 42 Trib 1 | Irregular Geometry | | | 0.035 | >1788 | Confluence with Unknown 1 to IH-10 | FPN42T1_1 | >100 |

Table B-6. Channel Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 42 Trib 1 | Irregular Geometry | | | 0.031 | >462 | 700 ft US of XS 2.0 to confluence with Unknown 1 | FPN42T1_2 | >100 |
| Flow Path No. 42 Trib 1 | Irregular Geometry | | | 0.03 | >462 | 600 ft US of XS 3.0 to 700 ft US of XS 2.0 | FPN42T1_2 | >100 |
| Flow Path No. 42 Trib 1 | Irregular Geometry | | | 0.032 | >462 | 600 ft US of XS 4.0 to 600 ft US of XS 3.0 | FPN42T1_2 | >100 |
| Flow Path No. 42 Trib 1 | Irregular Geometry | | | 0.033 | >462 | US of 600 ft US XS 4.0 | FPN42T1_2 | >100 |
| Flow Path No. 42A | Irregular Geometry | | | 0.035 | >197 | US of IH-10 | FPN42A | >100 |
| Flow Path No. 43 | Irregular Geometry | | | 0.035 | >1090 | 600 ft US of XS 1.0 to IH-10 | FPN43 | >100 |
| Flow Path No. 43 | Irregular Geometry | | | 0.031 | >1090 | 600 ft US of XS 2.0 to 600 ft US of XS 1.0 | FPN43 | >100 |
| Flow Path No. 43 | Irregular Geometry | | | 0.035 | >1090 | 600 ft US of XS 3.0 to 600 ft US of XS 2.0 | FPN43 | >100 |
| Flow Path No. 43 | Irregular Geometry | | | 0.035 | >1090 | US of 600 ft US XS 3.0 | FPN43 | >100 |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.03 | 660 | DS of Doniphan Dr. | N/A | 2 |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.031 | >6201 | A P Ramirez St to Doniphan Dr. | N/A | >100 |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.035 | 1020 | Kiely Rd. to A P Ramirez St. | N/A | 4 |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.03 | 2910 | IH-10 On-Ramp to Kiely Rd. | N/A | 10 |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.035 | >6070 | IH-10 to IH-10 On-Ramp | N/A | >100 |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.035 | 1000 | Tom Mays Dr. to IH-10 Off-Ramp | N/A | 4 |

Table B-6. Channel Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.031 | >2909 | 600 US XS 47.0 to Tom Mays Dr. | N/A | >100 |
| Flow Path No. 45 (Vinton) | Irregular Geometry | | | 0.03 | >511 | US of 600 ft US XS 47.0 | N/A | >100 |
| Flow Path No. 45A | Irregular Geometry | | | 0.032 | >1050 | Kiely Rd to confluence with FP45 | N/A | >100 |
| Flow Path No. 45A | Irregular Geometry | | | 0.032 | >1050 | Iron Dr. to Kiely Rd. | N/A | >100 |
| Flow Path No. 45A | Irregular Geometry | | | 0.032 | 630 | 200 ft US of XS 14.0 to Iron Dr. | N/A | 20 |
| Flow Path No. 45A | Irregular Geometry | | | 0.03 | 550 | Lovena Way to 200 ft US XS 14.0 | N/A | 10 |
| Flow Path No. 45A | Irregular Geometry | | | 0.03 | >1050 | IH-10 to Lovena Way | N/A | >100 |
| Flow Path No. 45A | Irregular Geometry | | | 0.015 | 120 | US of IH-10 | N/A | 20 |
| High Ridge | 34 | 1:1 | 7 | 0.016 | >3950 | DS of Redd Rd. | HR_1 | >100 |
| High Ridge | 12 | 1:1 | 4 | 0.016 | >2187 | Confluence with Arroyo 1 to Redd Rd. | HR_2 | >100 |
| High Ridge | 6 | 1:1 | 4 | 0.016 | >429 | US of confluence with Arroyo 1 | HR_3 | >100 |
| Montoya Drain | 35 | 0.4:1 | 6.5 | 0.03 | 1930 | Frontera Rd. to outlet | MD_1 | 80 |
| Montoya Drain | 36 | 1.8:1 & 1.3:1 | 6.5 | 0.035 | 1655 | Confluence with White Spur Drain to Frontera Rd. | MD_1 | 50 |
| Montoya Drain | 32 | 1.5:1 | 7 | 0.035 | >1090 | Country Club Pl. to confluence with White Spur Drain | MD_2 | >100 |
| Montoya Drain | 25 | 1.4:1 | 9 | 0.035 | >1049 | Redd Rd. to Country Club Pl. | MD_3 | >100 |
| Montoya Drain | 25 | 0.8:1 | 9 | 0.035 | >1049 | US of Redd Rd. | MD_3 | >100 |
| Ojo de Agua | Irregular Geometry | | | 0.02 | >3412 | DS of Lakehurst Rd. | ODA_1 | >100 |

Table B-6. Channel Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|---|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Ojo de Agua | 20 | 2:1 | 6.5 | 0.016 | >3412 | Resler Dr. to Lakehurst Rd. | ODA_1 | >100 |
| Ojo de Agua | 20 | 1:1 | 16 | 0.016 | >3412 | Westwind Dr. to Resler Dr. | ODA_1 | >100 |
| Ojo de Agua | 19 | 1.33:1 | 9 | 0.016 | >3412 | Confluence with Via Serena to Westwind Dr. | ODA_1 | >100 |
| Ojo de Agua | 10 | 1.33:1 | 4.5 | 0.016 | >785 | Via de La Paz Dr. to confluence with Via Serena | ODA_2 | >100 |
| Ojo de Agua | 12 | 0.82:1 | 5.5 | 0.016 | >785 | US of Via de La Paz Dr. | ODA_2 | >100 |
| Resler Channel | Irregular Geometry | | | 0.035 | >3025 | 450 ft US XS 1.0 to confluence with FPN38 | RC_1 | >100 |
| Resler Channel | Irregular Geometry | | | 0.033 | >3025 | IH-10 to 450 ft US XS 1.0 | RC_1 | >100 |
| Resler Channel | 26 | 1:1 | 12.5 | 0.016 | >3025 | Northwestern Dr. to IH-10 | RC_1 | >100 |
| Resler Channel | 20 | 1:1 | 8.75 | 0.016 | >3025 | 500 ft US XS 4.0 to Northwestern Dr. | RC_1 | >100 |
| Resler Channel | Irregular Geometry | | | 0.035 | >3025 | Confluence with FPN39A to 500 ft US of XS 4.0 | RC_1 | >100 |
| Resler Channel | Irregular Geometry | | | 0.034 | >1799 | US of confluence with FPN39A | RC_2 | >100 |
| Ridge View | 24 | 1:1 | 5 | 0.016 | >1285 | Desert Canyon Dr. to confluence with High Ridge | RV_1 | >100 |
| Ridge View | 15 | 1:1 | 5.5 | 0.016 | >1285 | Franklin Hills St. to Desert Canyon Dr. | RV_1 | >100 |
| Ridge View | Irregular Geometry | | | 0.035 | >1285 | US of Franklin Hills St. | RV_1 | >100 |
| Unknown 1 | Irregular Geometry | | | 0.035 | >335 | 850 ft US XS 1.0 to confluence with FPN42T1 | UN01 | >100 |

Table B-6. Channel Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Unknown 1 | Irregular Geometry | | | 0.033 | >335 | 1000 ft US XS 2.0 to 850 ft US XS 1.0 | UN01 | >100 |
| Unknown 1 | Irregular Geometry | | | 0.031 | >335 | US of 1000 ft US XS 2.0 | UN01 | >100 |
| Unknown 2 | Irregular Geometry | | | 0.03 | >395 | Confluence with UN03 to IH-10 | UN02_1 | >100 |
| Unknown 2 | 0 | 2:1 & 3:1 | 4 | 0.035 | >78 | 500 ft US XS 2.0 to confluence with UN03 | UN02_2 | >100 |
| Unknown 2 | Irregular Geometry | | | 0.035 | >78 | US of 500 ft US XS 2.0 | UN02_2 | >100 |
| Unknown 3 | Irregular Geometry | | | 0.032 | >238 | 600 ft US XS 1.0 to confluence with UN02 | UN03 | >100 |
| Unknown 3 | Irregular Geometry | | | 0.032 | >238 | 600 ft US XS 2.0 to 600 ft US XS 1.0 | UN03 | >100 |
| Unknown 3 | Irregular Geometry | | | 0.035 | >238 | US of 600 ft US XS 2.0 | UN03 | >100 |
| Unnamed Stream No. 23A | Irregular Geometry | | | 0.035 | >71 | US of IH-10 | UN23A | >100 |
| Unknown 24 | Irregular Geometry | | | 0.03 | >133 | US of IH-10 | UN024_1 | >100 |
| Unnamed Stream No. 24A | Irregular Geometry | | | 0.031 | >385 | 500 ft US XS 1.0 to IH-10 | UN24A_1 | >100 |
| Unnamed Stream No. 24A | Irregular Geometry | | | 0.03 | >385 | Confluence with UN24AT1 to 500 ft US XS 1.0 | UN24A_1 | >100 |
| Unnamed Stream No. 24A | Irregular Geometry | | | 0.03 | >113 | 1000 ft US XS 2.0 to confluence with UN24AT1 | UN24A_2 | >100 |
| Unnamed Stream No. 24A | Irregular Geometry | | | 0.035 | >113 | US of 1000 ft US XS 2.0 | UN24A_2 | >100 |
| Unnamed Stream No. 24A Trib 1 | Irregular Geometry | | | 0.03 | >70 | 500 ft US XS 1.0 to confluence with UN24A | UN24A_T1 | >100 |

Table B-6. Channel Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|---|--------------------|-------------------|------------|-------------|------------------------------|----------------------------------|-------------|--------------------------------------|
| Channel | Bottom Width (ft) | Side Slopes (H:V) | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Unnamed Stream No. 24A Trib 1 | Irregular Geometry | | | 0.031 | >70 | US of 500 ft US XS 1.0 | UN24A_T1 | >100 |
| Unnamed Stream No. 24B | Irregular Geometry | | | 0.03 | >290 | 1000 ft US XS 1.0 to IH-10 | UN24B | >100 |
| Unnamed Stream No. 24B | Irregular Geometry | | | 0.031 | >290 | US of 1000 ft US XS 1.0 | UN24B | >100 |
| Unnamed Stream No. 24C | Irregular Geometry | | | 0.035 | >138 | US of IH-10 | UN24C | >100 |
| Unnamed Stream No. 24D | Irregular Geometry | | | 0.035 | >54 | US of IH-10 | UN24D | >100 |
| White Spur Drain | 10 | 1.6:1 | 8 | 0.016 | >832 | Sunset Dr. to confluence with MD | WSD_1 | >100 |
| White Spur Drain | 10 | 1.6:1 | 6 | 0.016 | >832 | 375 ft US XS 2.0 to Sunset Dr. | WSD_1 | >100 |
| White Spur Drain | 8 | 1.3:1 | 6 | 0.016 | >1029 | Doniphan Dr. to 375 ft US XS 2.0 | WSD_1 | >100 |
| White Spur Drain | 6 | 1.25:1 | 2.8 | 0.016 | 472 | US of Doniphan Dr. | WSD_2 | 40 |

Table B-7. Channel Capacity Summary - West Central Region

| WEST CENTRAL REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|--|--------------------|-------------|------------|-------------|------------------------------|---|-------------|--------------------------------------|
| Channel/Cross-Section | Bottom Width (ft) | Side Slopes | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Canterbury Channel | Irregular Geometry | | | 0.026 | >1009 | 200 ft US XS 1.0 to outlet | CBC_1 | >100 |
| Canterbury Channel | Irregular Geometry | | | 0.026 | >1009 | IH-10 to 200 ft US XS 1.0 | CBC_1 | >100 |
| Canterbury Channel | Irregular Geometry | | | 0.034 | >1009 | 200 ft US XS 3.0 to IH-10 | CBC_1 | >100 |
| Canterbury Channel | Irregular Geometry | | | 0.033 | >1009 | 200 ft US XS 4.0 to 200 ft US XS 3.0 | CBC_1 | >100 |
| Canterbury Channel | 12.5 | Vertical | 9 | 0.016 | >1009 | Mesa St. to 200 ft US XS 4.0 | CBC_1 | >100 |
| Canterbury Channel | 11 | 1.5:1 | 7 | 0.016 | >1009 | Ridgecrest Dr. to Mesa St. | CBC_1 | >100 |
| Canterbury Channel | 6.5 | 1:1 | 3.5 | 0.016 | >1009 | 300 ft US XS 7.0 to Ridgecrest Dr. | CBC_1 | >100 |
| Canterbury Channel | 6.5 | 1:1 | 4 | 0.016 | >1009 | Stanton St. to 300 ft US XS 7.0 | CBC_1 | >100 |
| Canterbury Channel | Irregular Geometry | | | 0.03 | >1009 | US of Stanton St. | CBC_1 | >100 |
| Flow Path No. 20 | Irregular Geometry | | | 0.035 | 2700 | 650 ft US of IH-10 to Rio Grande | FPN20_1 | 10 |
| Flow Path No. 20 | Irregular Geometry | | | 0.016 | >6318 | 500 ft US XS 2.0 to 650 ft US IH-10 | FPN20_1 | >100 |
| Flow Path No. 20 | Irregular Geometry | | | 0.016 | >6318 | Confluence with Paragon Channel to 500 ft US XS 2.0 | FPN20_1 | >100 |
| Flow Path No. 20 | Irregular Geometry | | | 0.016 | >2949 | IH-10 to confluence with Paragon Channel | FPN20_2 | >100 |
| Flow Path No. 20 | Irregular Geometry | | | 0.034 | >2949 | 600 ft US XS 5.0 to IH-10 | FPN20_2 | >100 |
| Flow Path No. 20 | Irregular Geometry | | | 0.03 | >2949 | 500 ft US XS 6.0 to 600 ft US XS 5.0 | FPN20_2 | >100 |
| Flow Path No. 20 | Irregular Geometry | | | 0.03 | >2949 | Mesa Park to 500 ft US XS 6.0 | FPN20_2 | >100 |
| Flow Path No. 20 | Irregular Geometry | | | 0.04 | >2949 | Mesa St. to Mesa Park | FPN20_2 | >100 |
| Flow Path No. 21 | 13 | Vertical | 5 | 0.016 | 1375 | 300 ft US of Paisano to Rio Grande | FPN21_1 | 10 |

Table B-7. Channel Capacity Summary - West Central Region (Continued)

| WEST CENTRAL REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|--|--------------------|-------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel/Cross-Section | Bottom Width (ft) | Side Slopes | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 21 | Irregular Geometry | | | 0.03 | >3434 | Confluence with Canterbury Channel to 300 ft US of Paisano Dr. | FPN21_1 | >100 |
| Flow Path No. 21 | Irregular Geometry | | | 0.039 | >2293 | IH-10 to confluence with Canterbury Channel | FPN21_2 | >100 |
| Flow Path No. 21 | Irregular Geometry | | | 0.045 | >2293 | 1000 ft US XS 4.0 to IH-10 | FPN21_2 | >100 |
| Flow Path No. 21 | Irregular Geometry | | | 0.03 | >2293 | Okeefe Dr. to 1000 ft US XS 4.0 | FPN21_2 | >100 |
| Flow Path No. 21 | Irregular Geometry | | | 0.029 | >2293 | US of Okeefe (Wallington) Dr. | FPN21_2 | >100 |
| Flow Path No. 23 | Irregular Geometry | | | 0.035 | >3319 | 150 ft US of Paisano Dr. to Rio Grande | FPN23_1 | >100 |
| Flow Path No. 23 | Irregular Geometry | | | 0.037 | >3319 | Schuster Ave. to 150 ft US of Paisano Dr. | FPN23_1 | >100 |
| Flow Path No. 23 | Irregular Geometry | | | 0.031 | >3319 | Hawthorne St. to Schuster Ave. | FPN23_1 | >100 |
| Flow Path No. 23 | Irregular Geometry | | | 0.05 | >3319 | University Ave. to Hawthorne St. | FPN23_1 | >100 |
| Flow Path No. 23 | 27 | Vertical | 10 | 0.03 | >3319 | Mesa St. to University Ave. | FPN23_1 | >100 |
| Flow Path No. 23 | 16 | Vertical | 4 | 0.03 | 940 | Campbell St. to Mesa St. | FPN23_1 | 7 |
| Flow Path No. 23 | Irregular Geometry | | | 0.039 | >3319 | Virginia St. to Campbell St. | FPN23_1 | >100 |
| Flow Path No. 23 | Irregular Geometry | | | 0.048 | >3319 | US of Virginia St. | FPN23_1 | >100 |
| Paragon Channel | Irregular Geometry | | | 0.016 | >2975 | 350 Ds XS 2.0 to confluence with FPN20 | PC_1 | >100 |
| Paragon Channel | Irregular Geometry | | | 0.032 | >2975 | IH-10 to 350 ft DS XS 2.0 | PC_1 | >100 |
| Paragon Channel | Irregular Geometry | | | 0.029 | >2975 | 400 ft US XS 3.0 to IH-10 | PC_1 | >100 |
| Paragon Channel | Irregular Geometry | | | 0.03 | >2975 | 500 ft US XS 4.0 to 400 ft US XS 3.0 | PC_1 | >100 |
| Paragon Channel | Irregular Geometry | | | 0.034 | >2975 | 500 ft DS XS 6.0 to 500 ft US XS 4.0 | PC_1 | >100 |
| Paragon Channel | 9 | 1:1 | 7 | 0.016 | >2975 | Mesa St. to 500 ft DS XS 6.0 | PC_1 | >100 |
| Paragon Channel | 11 | 0.07:1 | 10 | 0.016 | >2975 | 530 ft US of XS 7.0 to Mesa St. | PC_1 | >100 |

Table B-7. Channel Capacity Summary - West Central Region (Continued)

| WEST CENTRAL REGION - CHANNEL CAPACITY SUMMARY | | | | | | | | |
|--|--------------------|-------------|------------|-------------|------------------------------|--|-------------|--------------------------------------|
| Channel/Cross-Section | Bottom Width (ft) | Side Slopes | Depth (ft) | Manning's n | Cross Section Capacity (cfs) | Cross Section Location | HMS Node ID | Interpolated Return Interval (years) |
| Paragon Channel | Irregular Geometry | | | 0.034 | >2975 | 700 ft DS of Stanton St. to 530 ft US XS 7.0 | PC_1 | >100 |
| Paragon Channel | 11 | 0.7:1 | 10 | 0.016 | >2975 | Stanton St. to 700 ft DS of Stanton St. | PC_1 | >100 |
| Paragon Channel | Irregular Geometry | | | 0.033 | >2975 | US of Stanton St. | PC_1 | >100 |

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Table B-8. Culvert Capacity Summary - Central Region

| CENTRAL REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|--------------------------------------|-------------|-----------------|------------------------|----------------------|----------------------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Copia Ditch | 4-56 x 48-inch Box | 45 | 7.29 | 405.77 | Louisville Ave | J_Copia_Ditch_US | 50 |
| Copia Ditch | 5-56 x 48-inch Box | 45 | 7.99 | 546.32 | Richmond Ave | J_Copia_Ditch_US | 50 |
| Copia Ditch | 3-56 x 48-inch Box | 45 | 8.78 | 343.79 | Lebanon Ave | J_Copia_Ditch_US | 50 |
| Copia Ditch | 3-76" x 48" CBC for 3-76" x 54" Arch | 45 | 14.06 | 642.19 | Savannah Ave | J_Copia_Ditch_US | 50 |
| Copia Ditch | 3-36-inch Circular | 56 | 14.31 | 202.36 | Altura Ave | J_Copia_Ditch_US | 50 |
| Copia Ditch | 1-24-inch Circular | 13 | 4.95 | 11.66 | 50' NE of RR Tracks | J_Copia_Ditch_DS | 50 |
| Copia Ditch | 1-72" x 43" CBC for 1-72" x 48" Arch | 37 | 12.12 | 74.91 | RR Tracks | A_Memorial_Park | 50 |
| Paisano Ditch | 1-72" x 43" CBC for 1-72" x 48" Arch | 340 | 7.55 | 41.46 | Piedras St | J_Nixon_Cypress_to_Central | 50 |
| Paisano Ditch | 2-24-inch Circular | 25 | 8.92 | 53.1 | 100' E of Piedras St | J_Nixon_Cypress_to_Central | 50 |
| Paisano Ditch | 1-60-inch Circular | 132 | 11.79 | 200.53 | San Marcial St | J_Nixon_Cypress_to_Central | 100 |
| Pollard Ditch | 2-8x3 ft Box | 20 | 14.72 | 288.81 | RR Tracks | A_Pollard Ditch | 58 |
| Van Buren Ditch | 6-6 x 5 ft Box | 110 | 10.28 | 1344.86 | Alabama St | J_Van Buren Ditch Inflow | 25 |
| Government Hills Channel | 2-10 x 4 ft Box | 76 | 11.31 | 339.59 | Altura Ave | J_Gov Hills North Inflow | 25 |
| Government Hills Channel | 2-8 x 4 ft Box | 45 | 11.46 | 276.03 | Hastings Drive | J_Gov Hills North Inflow | 10 |
| Government Hills Channel | 2-8 x 3 Box | 70 | 10.12 | 209.42 | Cambridge Ave | J_Gov Hills North Inflow | 10 |
| Government Hills Channel | 2-8 x 3 Box | 75 | 10.6 | 227.07 | Cumberland Ave | J_Gov Hills North Inflow | 10 |
| Government Hills Channel | 2-8 x 3 Box | 72 | 10.58 | 231.38 | Chester Ave | J_Gov Hills North Inflow | 10 |

Table B-8. Culvert Capacity Summary - Central Region (Continued)

| CENTRAL REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|---------------------|----------------|--------------------|------------------------------|------------------|--------------------------|---|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Government Hills Channel | 3-8 x 3 Box | 70 | 10.42 | 348.27 | Oxford Ave | J_Gov Hills North Inflow | 50 |
| Government Hills Channel | 5-5 x 2.5 ft Box | 70 | 8.03 | 201.92 | Trowbridge Ave | J_Gov Hills North Inflow | 10 |
| Government Hills Channel | 2-13.6 x 4.5 ft Box | 42 | 10.84 | 535.03 | Bliss Ave | J_Gov Hills North Inflow | 100 |
| Government Hills Channel | 2-8 x 4 ft Box | 502 | 13.27 | 353.92 | Hueco Ave | J_Gov Hills North Inflow | 50 |
| Government Hills Channel | 4-5.5 x 3.6 Box | 44 | 10.95 | 867.6 | Clifton Ave | J_Gov Hills North Inflow | 100 |
| Government Hills Channel | 2-12.6 x 4.8 Box | 42 | 12.32 | 1053.42 | LaLuz Ave | J_Gov Hills North Inflow | 100 |

Table B-9a. Culvert Capacity Summary - East Side Region

| EAST SIDE REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|------------------------|-------------|-----------------|------------------------|--------------------------|-------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Bluff Channel | 1-10 x 5 ft Box | 94 | 14 | 509 | Esther Lama Drive | 9J | 2 |
| Fort Bliss Spur Drain Channel | 1-7 x 4 ft Box | 97 | 17 | 304 | Airway Boulevard | A-PD-1 | 5 |
| Fort Bliss Spur Drain Channel | 1-8 x 4 ft Box | 168 | 5 | 144 | Robert E. Lee Road | A-PD-1 | 2 |
| Fort Bliss Spur Drain Channel | 1-92.4 x 65-inch Arch | 37 | 13 | 436 | Railroad Crossing | A-PD-1 | 10 |
| Fort Bliss Spur Drain Channel | 2-6 x 4 ft Box | 414 | 21 | 791 | IH-10 | A-PD-1 | >100 |
| Sunmount Channel | 2-4.5 x 3 ft Box | 154 | 10 | 258 | Viscount Boulevard | A_Sunmount | 30 |
| RV Channel | 6-6 x 4 ft Box | 141 | 18 | 2062 | Paseo Del Este Boulevard | 124A & 124E | >100 |
| RV Channel | 3-48-inch Circular | 103 | 14 | 390 | Pine Springs Drive | 124A & 124E | 10 |
| RV Channel | 5-6 x 3 ft Box | 141 | 8 | 729 | Rojas Drive | 124JW | 5 |
| RV Channel | 3-6 x 3 ft Box | 113 | 16 | 516 | RV Drive | 124JW | 5 |
| RV Channel | 3-6 x 3 ft Box | 40 | 8 | 414 | St. Francis Street | 124JW | 5 |
| Mercantile Channel | 2-42 ft Conspan Bridge | 256 | | | Paseo Del Este Boulevard | 124C | >100 |
| Mercantile Channel | 4-54-inch Circular | 146 | 16 | 949 | Mercantile Avenue | 124C | 40 |
| Mercantile Channel | 3-60-inch Circular | 134 | 17 | 718 | Rojas Drive | 124JE | 10 |
| Joe Battle Channel | 4-6 x 4 ft Box | 61 | 12 | 1121 | Painted Quail Place | A-AM-5 | >100 |
| Peter Hurd Channel | 1-6 x 4 ft Box | 127 | 17 | 189 | Vista Del Sol Drive | A-AM-7 | >100 |
| Lorne Channel | 1-8 x 2 ft Box | 70 | 4 | 59 | Lorne Road | A-PD-6 | 10 |

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Table B-9b. Culvert Capacity Summary - East Side Region - IH-10 Corridor

| EAST SIDE REGION - IH-10 CORRIDOR CULVERT CAPACITY SUMMARY | | | | | | | |
|--|-----------|-------------|--------|-------|------------|-----------------------------|-------------|
| Watershed ID | Sta | Description | Length | Slope | Qcap (cfs) | Q _{100exp} (cfs) * | At Capacity |
| PHELPS DODGE 1A | 89+00 | 24" RCP | 288.00 | 3.37% | 23.62 | 80.93 | No |
| | 97+19 | 24" RCP | 288.00 | 0.95% | 22.30 | 80.93 | No |
| | 103+08 | 36" RCP | 290.00 | 1.98% | 56.26 | 80.93 | No |
| | 108+12 | 24" RCP | 292.00 | 2.41% | 23.54 | 80.93 | No |
| EID 1 | 113+09 | 24" RCP | 296.00 | 2.03% | 23.50 | 34.70 | No |
| | 117+30 | 30" RCP | 290.00 | 2.68% | 38.03 | 34.70 | Yes |
| | 119+00 | 36" RCP | 332.57 | 2.06% | 76.26 | 34.70 | Yes |
| | 122+02 | 36" RCP | 410.30 | 2.71% | 85.56 | 34.70 | Yes |
| | 127+95 | 24" RCP | 296.00 | 3.25% | 23.61 | 34.70 | No |
| | 135+59 | 30" RCP | 292.00 | 2.73% | 38.04 | 34.70 | Yes |
| | 138+67 | 30" RCP | 294.00 | 2.20% | 37.95 | 34.70 | Yes |
| | 141+80 | 24" RCP | 292.00 | 1.51% | 23.46 | 34.70 | No |
| | 147+10 | 24" RCP | 292.00 | 1.92% | 23.50 | 34.70 | No |
| | 153+70.8 | 24" RCP | 290.00 | 2.46% | 23.54 | 34.70 | No |
| | 158+45 | 24" RCP | 292.00 | 2.45% | 23.54 | 34.70 | No |
| | 170+55 | 24" RCP | 338.26 | 2.86% | 34.25 | 47.67 | No |
| EID 2 | 172+14 | 36" RCP | 298.00 | 2.39% | 56.29 | 47.67 | Yes |
| | 176+42 | 24" RCP | 286.00 | 3.52% | 23.63 | 47.67 | No |
| | 182+70.4 | 30" RCP | 288.00 | 3.70% | 38.11 | 33.57 | Yes |
| RHL 1 | 187+74 | 3-30" RCP | 353.46 | 2.74% | 226.16 | 33.57 | Yes |
| | 197+15 | 24" RCP | 358.38 | 4.58% | 44.91 | 33.57 | Yes |
| | 209+30 | 24" RCP | 294.00 | 3.29% | 23.61 | 15.57 | Yes |
| RHL 2 | 212+31.10 | 24" RCP | 288.00 | 2.42% | 23.54 | 15.57 | Yes |
| | 216+35 | 24" RCP | 288.00 | 2.56% | 23.55 | 15.57 | Yes |
| | 221+63 | 2-30" RCP | 309.69 | 2.73% | 147.43 | 643.00 | No |
| EASTWOOD 2 | 235+09.12 | 2-30" RCP | 303.17 | 2.17% | 100.02 | 643.00 | No |
| | 237+13.77 | 3-36" RCP | 292.00 | 2.10% | 168.86 | 643.00 | No |
| | 255+40.88 | 2-24" RCP | 303.03 | 3.12% | 47.20 | 143.70 | No |
| STLRHL | 262+63.75 | 2-36" RCP | 293.83 | 1.25% | 156.00 | 143.70 | Yes |
| | 274+75 | 36" RCP | 318.67 | 3.47% | 108.94 | 143.70 | No |
| | 296+00 | 24" RCP | 290.00 | 1.51% | 23.46 | 102.75 | No |
| CAROLINA DAM 2 | 301+68 | 3-36" RCP | 302.32 | 2.44% | 213.65 | 102.75 | Yes |
| | 312+06.45 | 3-24" RCP | 305.05 | 1.83% | 76.43 | 142.30 | No |
| LOMALAND 10 | 322+25 | 24" RCP | 286.00 | 2.49% | 23.54 | 114.47 | No |
| | 326+50 | 24" RCP | 286.00 | 1.68% | 23.48 | 114.47 | No |
| | 333+02.88 | 3-24" RCP | 286.00 | 1.21% | 70.3 | 114.47 | No |
| | 337+58 | 4-30" RCP | 288.00 | 0.99% | 151.01 | 114.47 | Yes |
| | 347+61 | 10-30" RCP | 350.73 | 1.86% | 763.15 | 114.47 | Yes |
| | 367+12 | 30" RCP | 288.00 | 2.17% | 37.95 | 114.47 | No |
| | 370+50 | 24" RCP | 286.00 | 2.50% | 23.55 | 56.24 | No |
| MESA DRAIN 3 | 379+65 | 24" RCP | 290.00 | 1.47% | 23.46 | 56.24 | No |
| | 385+85 | 3-24" RCP | 295.97 | 2.17% | 69.77 | 56.24 | Yes |
| | 393+16 | 24" RCP | 336.12 | 2.04% | 29.21 | 56.24 | No |
| | 401+16 | 5-30" RCP | 303.15 | 1.64% | 272.12 | 56.24 | Yes |
| | 410+21 | 2-24" RCP | 287.00 | 2.40% | 47.07 | 56.24 | No |
| | 416+75 | 2-30" RCP | 297.88 | 2.62% | 109.95 | 56.24 | Yes |

**Table B-9b. Culvert Capacity Summary - East Side Region - IH-10 Corridor
(Continued)**

| EAST SIDE REGION - IH-10 CORRIDOR CULVERT CAPACITY SUMMARY | | | | | | | |
|--|-----------|-------------------|--------|-------|---------------|--------------------------------|----------------|
| Watershed ID | Sta | Description | Length | Slope | Qcap (cfs) | Q _{100exp} (cfs) * | At Capacity |
| MESA DRAIN 4 | 424+44 | 24" RCP | 286.00 | 4.58% | 23.72 | 48.40 | No |
| | 429+00 | 24" RCP | 290.00 | 5.17% | 23.77 | 48.40 | No |
| | 433+75 | 24" RCP | 286.00 | 6.22% | 23.83 | 48.40 | No |
| WS 9A | 436+50 | 24" RCP | 282.00 | 4.99% | 23.76 | 37.00 | No |
| | 449+33.24 | 24" RCP | 330.81 | 1.27% | 26.55 | 16.00 | Yes |
| WS 22 | 475+00 | 8'x4' CBC | 215.48 | 1.67% | 219.15 | 173.12 | Yes |
| | 482+00 | 54" RCP | 184.42 | 1.70% | 137.00 | 173.12 | No |
| | 487+00 | 42" RCP | 186.00 | 1.82% | 78.67 | 173.12 | No |
| | 498+83 | 6'x4' CBC | 161.67 | 2.31% | 164.36 | 173.12 | No |
| | 504+97 | 9'x5' CBC | 190.15 | 0.86% | 324.09 | 173.12 | Yes |
| WS 77 | 510+67 | 3'-5'x5' MBC | 164.66 | 1.51% | 540.15 | 129.75 | Yes |
| | 518+72 | 48" RCP | 164.00 | 0.88% | 105.04 | 129.75 | No |
| | 529+43 | 4-48" RCP | 336.00 | 2.64% | 422.15 | 129.75 | Yes |
| | 536+00 | 30" RCP | 324.00 | 3.34% | 38.11 | 129.75 | No |
| WS-90 | 542+18 | 30", 36", 42" RCP | 520.00 | 1.63% | 37.86 | 26.7 | Yes |
| WS-100 | 547+17 | 2-8'x5' MBC | 430.21 | 1.59% | 576.16 | 72.7 | Yes |
| | 553+19 | 42" RCP | 382.00 | 1.71% | 78.67 | 72.7 | Yes |
| WS-110 | 558+79 | 36" RCP | 338.00 | 2.31% | 56.29 | 37.93 | Yes |
| | 562+60 | 54" RCP | 334.00 | 2.10% | 137.15 | 37.93 | Yes |
| | 567+66 | 36" RCP | 332.00 | 2.29% | 56.29 | 37.93 | Yes |

* Q_{100exp} for each crossing was assumed as the complete watershed flow equally divided by the number of crossings in the watershed.

Table B-10. Culvert Capacity Summary - Mission Valley Region

| MISSION VALLEY REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|--|--------------------|-------------|-----------------|------------------------|--|-----------------------|------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | Channel Cross Section | Percent of Channel Capacity Passed |
| Playa Drain | 1-36-inch Circular | 47 | 18 | 124 | Playa Drain and Crossing just DS of Yarbrough | Playa_CS_3 | 15% |
| Playa Drain | 1-7 x 7 ft Box | 125 | 14 | 696 | Playa Drain and Zaragosa | Playa_CS_4 | >100% |
| Playa Drain | 1-10 x 9 ft Box | 400 | 15 | 988 | Playa Drain and Americas | Playa_CS_4 | >100% |
| Mesa Drain | 3-10 x 9 ft Box | 52 | 6 | 1571 | Mesa Drain and Bowman Irrigation Canal | Mesa_CS_1 | >100% |
| Mesa Drain | 3-10 x 8 ft Box | 60 | 12 | 2942 | Mesa Drain and Irrigation Canal DS of Yarbrough | Mesa_CS_2 | >100% |
| Mesa Drain | 4-10 x 10 ft Box | 40 | 5 | 2195 | Mesa Drain and Irrigation Canal at Zaragosa | Mesa_CS_3 | >100% |
| Mesa Drain | 4-10 x 9 ft Box | 80 | 4 | 1400 | Mesa Drain and Zaragosa | Mesa_CS_3 | >100% |
| Mesa Drain | 4-10 x 10 ft Box | 60 | 8 | 3242 | Mesa Drain and Irrigation Canal DS of Zaragosa (Near Earth #2) | Mesa_CS_3 | >100% |

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Table B-11. Culvert Capacity Summary - Northeast Region

| NORTHEAST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|--------------------|-------------|-----------------|------------------------|------------------------------|-------------------------------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Electric Ditch Diversion | 3-8 x 4 ft Box | 51 | 11 | 886 | Woodrow Bean | J_Electric Ditch (Area Reduction) | >100 |
| P.S.B. Channel No. 2 | 1-36-inch Circular | 91 | 12 | 77 | Sun Valley St | J_PSB Ch 2 at Rushing | 10 |
| P.S.B. Channel No. 2 | 2-48-inch Circular | 61 | 15 | 240 | Bon Aire St | J_PSB Ch 2 at Rushing | 10 |
| P.S.B. Channel No. 2 | 2-66-inch Circular | 90 | 18 | 494 | Kentworthy St | J_PSB Ch 2 at Rushing | 20 |
| P.S.B. Channel No. 2 | 3-8 x 3.6 ft Box | 36 | 12 | 765 | McCombs West | J_PSB Ch 2 D/S | 30 |
| P.S.B. Channel No. 2 | 3-8 x 5 ft Box | 40 | 11 | 1057 | McCombs East | J_PSB Ch 2 D/S | 60 |
| P.S.B. Channel No. 2 | 4-6 x 6 ft Box | 430 | 27 | 2638 | Gateway Freeway | J_PSB Ch 2 at Rushing | >100 |
| Diana Ditch | 3-8x3 ft Box | 62 | 10 | 441 | Maxwell Ave | A_Diana Ditch US Sunrise Ch | 20 |
| Diana Ditch | 3-8 x 4 ft Box | 76 | 9 | 514 | Alps Dr | A_Diana Ditch US Sunrise Ch | 20 |
| Diana Ditch | 4-8 x 4 ft Box | 62 | 11 | 1360 | Blueridge Dr | A_Diana Ditch US Sunrise Ch | >100 |
| Diana Ditch | 4-9 x 4 ft Box | 60 | 12 | 999 | Guadalupe Dr | J_Diana Ditch U/S | 60 |
| War Road Channel | 1-8 x 5 ft Box | 102 | 16 | 299 | Marcus Uribe and MLK Jr Blvd | J_War Road Channel (Area Reduction) | >100 |
| Diana Ditch | 4-6 x 6 ft Box | 90 | 11 | 1098 | Tetons Dr | J_Diana Ditch D/S | 10 |
| Diana Ditch | 4-10 x 3 ft Box | 88 | 9 | 794 | Hercules Ave | J_Diana Ditch D/S | 10 |
| Diana Ditch | 5-10 x 3 ft Box | 53 | 9 | 1014 | Titanic Dr | J_Diana Ditch D/S | 10 |
| Diana Ditch | 5-10 x 3 ft Box | 53 | 9 | 1012 | Atlas Ave/Luis Dr | J_Diana Ditch D/S | 10 |
| Diana Ditch | 6-7 x 4 ft Box | 267 | 10 | 1123 | Railroad Dr | J_Diana Ditch and FB Div | 10 |
| Diana Ditch | 5-10 x 3 ft Box | 62 | 9 | 1045 | Vulcan Ave | J_Diana Ditch D/S | 10 |
| Statler Ditch (AKA Mt. Everest Channel) | 2-5 x 2 ft Box | 81 | 6 | 71 | Tetons Dr | A_Statler Ditch | 10 |
| Statler Ditch (AKA Mt. Everest Channel) | 2-5 x 2 ft Box | 81 | 6 | 71 | Hercules Ave | A_Statler Ditch | 10 |
| Statler Ditch (AKA Mt. Everest Channel) | 1-7 x 4 ft Box | 131 | 9 | 180 | Railroad Dr | J_RR Ditch at Statler Ditch | 10 |

Table B-11. Culvert Capacity Summary - Northeast Region (Continued)

| NORTHEAST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|--------------------|-------------|-----------------|------------------------|---------------------------------|---|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| War Road Channel | 2-9 x 6 ft Box | 140 | 12 | 956 | John Cunningham and MLK Jr Blvd | J_War Road Channel | >100 |
| Statler Ditch (AKA Mt. Everest Channel) | 1-66-inch Circular | 33 | 14 | 304 | Railroad Tracks | J_RR Ditch at Statler Ditch | 10 |
| Threadgill | 1-8 x 5 ft Box | 79 | 12 | 281 | Sanders Ave | J_Tobin Drain U/S Army Ditch | 10 |
| Threadgill | 3-6 x 3 ft Box | 47 | 7 | 315 | Wren Dr | J_Tobin Drain U/S Army Ditch | 10 |
| Threadgill | 5-6 x 3 ft Box | 52 | 7 | 527 | Joe Herrera Dr | J_Tobin Drain D/S | 10 |
| Threadgill | 5-6 x 3 ft Box | 63 | 3 | 283 | Raymond Telles Dr | J_Tobin Drain D/S | 10 |
| Threadgill | 5-6 x 3 ft Box | 50 | 6 | 506 | Alps Dr | J_Tobin Drain D/S | 10 |
| Threadgill | 5-6 x 3 ft Box | 78 | 8 | 539 | Hollings St | J_Tobin Drain D/S | 10 |
| Threadgill | 4-6 x 3 ft Box | 94 | 9 | 480 | Hondo Pas Ave | J_Tobin Drain D/S | 10 |
| Threadgill | 4-7 x 6.5 ft Box | 112 | 13 | 2318 | Railroad Dr | J_RR Drain U/S of Tobin | >100 |
| Threadgill | 3-96-inch Circular | 30 | 18 | 2769 | Railroad Tracks | J_Tobin Drain D/S | 100 |
| War Road Channel | 1-5 x 2 ft Box | 68 | 12 | 57 | Loma Clara and MLK Jr Blvd | J_War Road Channel (Area Reduction) | >100 |
| Northgate Outlet Channel | 2-10 x 6 ft Box | 72 | 9 | 1020 | Diana Dr | J_Northgate Dam Out | >100 |
| Northgate Outlet Channel | 1-10 x 6 ft Box | 493 | 22 | 510 | W of Gateway South | J_Northgate Dam Out | >100 |
| Railroad Dr | 1-18-inch Circular | 100 | 7 | 11 | Falcon Ave | J_RR Drain U/S of Tobin (Area Reduction) | 10 |
| Railroad Dr | 1-12-inch Circular | 109 | 8 | 6 | Waycross Ave | J_RR Drain U/S of Tobin (Area Reduction) | 10 |
| Railroad Dr | 1-18-inch Circular | 127 | 10 | 12 | Wren Dr | J_RR Drain U/S of Tobin (Area Reduction) | 10 |
| Railroad Dr | 1-18-inch Circular | 114 | 7 | 10 | Lexington Dr | J_RR Drain U/S of Tobin (Area Reduction) | 10 |
| Railroad Dr | 1-8 x 4 ft Box | 905 | 10 | 262 | McCombs St | J_RR Drain U/S of Tobin (Area Reduction) | >100 |
| Railroad Dr | 5-8 x 4 ft Box | 45 | 12 | 360 | Julian Ave | J_RR Ditch Downstream | 10 |
| Railroad Dr | 1-12-inch Circular | 23 | 10 | 5 | South of Falcon Ave | J_RR Drain U/S of Tobin (Area Reduction) | 10 |
| War Road Channel | 1-36-inch Circular | 149 | 12 | 83 | Loma Real and MLK Jr Blvd | J_War Road Channel (Area Reduction) | >100 |

Table B-11. Culvert Capacity Summary - Northeast Region (Continued)

| NORTHEAST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|--------------------|-------------|-----------------|------------------------|---------------------|-------------------------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Bossworth (AKA Clear View Channel) | 3-36-inch Circular | 61 | 11 | 149 | Morningside Dr | A_Bossworth U/S | 10 |
| Bossworth (AKA Clear View Channel) | 3-36-inch Circular | 67 | 10 | 201 | Byron St | A_Bossworth U/S | 10 |
| Bossworth (AKA Clear View Channel) | 3-6 x 4 ft Box | 101 | 19 | 739 | Dyer | J_Bossworth Ch D/S | 100 |
| Western Freeway Channel | 9-12.5 X 6 Box | 167 | 8 | 5609 | McComb Rd | J_W Fwy D/S | >100 |
| P.S.B. Channel No. 2 | 1-48-inch Circular | 440 | 8 | 103 | Gateway Apt | J_PSB Ch 2 at Rushing | 10 |
| Range Dam Outlet (Army Ditch) | 3-6 x 3 ft Box | 119 | 12 | 316 | Dyer Street | J_Army Ditch (Area Reduction) | >100 |
| Range Dam Outlet (Army Ditch) | 1-2 x 2 ft Box | 49 | 11 | 43 | Raymond Telles Dr | J_Army Ditch | 10 |
| Johnson Channel | 1-48-inch Circular | 66 | 8 | 50 | Truman Ave. | A_Johnson Channel | 10 |
| Clearview Channel | 4-60-inch Circular | 147 | 21 | 1007 | Alabama Ave. | A_Bossworth U/S | >100 |
| P.S.B. Channel No. 1 | 6-9 x 7 ft Box | 445 | 30 | 3627 | Gateway South US 54 | J_PSB Ch1 Jct2 | >100 |
| P.S.B. Channel No. 1 | 4-8 x 10 ft Box | 44 | 20 | 3620 | McCombs Rd | J_PSB Ch1 Jct3 | >100 |

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Table B-12. Culvert Capacity Summary - Northwest Region

| NORTHWEST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|---|-------------|-----------------|------------------------|---------------------|-------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Arroyo 4 | 2-42-inch Circular | 80 | 28 | >114 | Broadmoor St | A4_1 | >100 |
| Arroyo 4 | 1-30-inch Circular and 1-36-inch Circular | 95 | 24 | 192 | Westwind Dr | A4_1 | 10 |
| Arroyo 4 | 1-54-inch Circular | 172 | 29 | 296 | Northwind Dr | A4_1 | 25 |
| Arroyo 4 | 2-42-inch Circular | 164 | 30 | 582 | El Puente St | A4_1 | 40 |
| Arroyo 4 | 2-48-inch Circular | 1677 | 18 | 442 | Resler Dr | A4_1 | 25 |
| Arroyo 4 | 2-4 x 4 ft Box | 484 | 24 | 415 | Mesa St | A4_1 | 10 |
| Arroyo 4 | no existing information | NA | NA | NA | Desert Tr | A4_1 | NA |
| Arroyo 4 | no existing information | NA | NA | NA | Gem St | A4_1 | NA |
| Arroyo 4 | no existing information | NA | NA | NA | IH-10 | A4_1 | >100 |
| Arroyo 5 | 1-6 x 4 ft Box | 478 | 47 | 251 | Mesa St | A5_1 | 7 |
| Arroyo 5 | 2-6 x 6 ft Box | 231 | 24 | >1519 | Resler Dr | A5_1 | >100 |
| Arroyo 5 | no existing information | NA | NA | NA | IH-10 | A5_1 | NA |
| Doniphan Ditch | 2-35 x 24-inch Arch | 54 | 7 | 52 | Bird Rd | DD_1 | 6 |
| Doniphan Ditch | 2-35 x 24-inch Arch | 64 | 8 | 61 | Frontera Rd | DD_1 | 4 |
| Doniphan Ditch | 2-6 x 4 ft Box | 121 | 10 | 392 | Sunland Park Rd | DD_1 | 25 |
| Doniphan Ditch | 2-36-inch Circular | 111 | 7 | 62 | Dona Ana County Rd | DD_1 | 2 |
| Doniphan Ditch | no existing information | NA | NA | NA | Railroad 2 | DD_1 | NA |
| Doniphan Ditch | 2-36-inch Circular | 48 | 9 | 108 | Power Station | DD_1 | 3 |
| Doniphan Ditch | no existing information | NA | NA | NA | Railroad 1 | DD_1 | NA |
| Doniphan Ditch | no existing information | NA | NA | NA | Pedestrian Overpass | DD_1 | NA |
| Flow Path No. 38 | 1-24-inch Circular | 112 | 10 | 29 | Playa del Sol St | FPN38_3 | 4 |
| Flow Path No. 38 | 1-36-inch Circular | 162 | 17 | 47 | Corona del Sol St | FPN38_3 | 5 |
| Flow Path No. 38 | 2-6 x 4 ft Box | 81 | 15 | 365 | Villa del Sol St | FPN38_3 | 30 |
| Flow Path No. 38 | 6-10.5 x 3.5 ft Box | 530 | 2 | >46 | Resler Dr | FPN38_1US | >100 |
| Flow Path No. 38 | 3-8 x 7 ft Box | 184 | 9 | >896 | Northwestern Dr | FPN38_1US | >100 |

Table B-12. Culvert Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|-----------------------------------|-------------|-----------------|------------------------|------------------|--------------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 38 | 232 ft wide Bridge | 142 | 4 | >896 | IH-10 | FPN38_1US | >100 |
| Flow Path No. 39A | 4-15 x 5 ft Box | 212 | 16 | >2498 | Resler Dr | FPN39A2US | >100 |
| Flow Path No. 39A | 110.7 ft wide Bridge | 137 | 17 | >2509 | IH-10 | FPN39A1US | >100 |
| Flow Path No. 39A Redirect | 1-23 x 9 ft Arch | 126 | 19 | 2114 | Northwestern Dr | FPN40_2US | 50 |
| Flow Path No. 40 | 4-6 x 6 ft Box and 3-7 x 4 ft Box | 155 | 17 | 1517 | Transmountain Rd | FPN40_3 | 60 |
| Flow Path No. 40 | 150 ft wide Bridge | 396 | 18 | >4361 | IH-10 | FPN40_2US, FPN40_3 | >100 |
| Flow Path No. 42 | 114 ft wide Bridge | 154 | 12 | >937 | IH-10 | FPN42 | >100 |
| Flow Path No. 42A | 2-54-inch Circular | 351 | 15 | >197 | IH-10 | FPN42A | >100 |
| Flow Path No. 43 | 95 ft wide Bridge | 140 | 10 | >1090 | IH-10 | FPN43 | >100 |
| Flow Path No. 45 | 13-9 x 5 ft Box | 39 | 17 | 5065 | IH-10 Off-Ramp | NA (HEC-RAS) | 50 |
| Flow Path No. 45 | 158 ft wide Bridge | 142 | 8 | >6070 | IH-10 | NA (HEC-RAS) | 100 |
| Flow Path No. 45 | 13-9 x 5.3 ft Box | 42 | 7 | 4610 | IH-10 On-Ramp | NA (HEC-RAS) | 40 |
| Flow Path No. 45 | Low Water Crossing | 69 | NA | NA | Quejette Rd | NA (HEC-RAS) | NA |
| Flow Path No. 45 | 2-8 x 3 ft Box | 43 | 11 | 303 | Kiely Rd | NA (HEC-RAS) | 1 |
| Flow Path No. 45 | Low Water Crossing | 32 | NA | NA | Vinton Rd | NA (HEC-RAS) | NA |
| Flow Path No. 45 | 4-36-inch Circular | 67 | 16 | 348 | Ap Ramirez St | NA (HEC-RAS) | 1 |
| Flow Path No. 45 | 2-6 x 6 ft Box | 70 | 16 | 915 | Doniphan Dr | NA (HEC-RAS) | 3 |
| Flow Path No. 45 | 68 ft wide Bridge | 19 | 16 | 3555 | Railroad | NA (HEC-RAS) | 20 |
| Flow Path No. 45 | no existing information | NA | NA | NA | Tom Mays Dr. | NA (HEC-RAS) | NA |
| Flow Path No. 45 | no existing information | NA | NA | NA | De Alva Dr. | NA (HEC-RAS) | NA |
| Flow Path No. 45A | 3-54-inch Circular | 341 | 16 | >189 | IH-10 | NA (HEC-RAS) | >100 |
| Flow Path No. 45A | 5-48-inch Circular | 73 | 17 | >788 | Lovena Way | NA (HEC-RAS) | >100 |
| Flow Path No. 45A | 2-30-inch Circular | 61 | 11 | 66 | Railroad 4 | NA (HEC-RAS) | 1 |
| Flow Path No. 45A | 1-30 x 48-inch Ellipse | 128 | 13 | 73 | Railroad 3 | NA (HEC-RAS) | 1 |
| Flow Path No. 45A | 1-24-inch Circular | 72 | 8 | 22 | Railroad 2 | NA (HEC-RAS) | 0 |
| Flow Path No. 45A | 1-48-inch Circular | 76 | 13 | 115 | Railroad 1 | NA (HEC-RAS) | 2 |

Table B-12. Culvert Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|-------------------------|-------------|-----------------|------------------------|---------------------|--------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 45A | 3-30-inch Circular | 38 | 9 | 116 | Iron Dr | NA (HEC-RAS) | 2 |
| Flow Path No. 45A | 2-30-inch Circular | 47 | 8 | 71 | Kiely Rd | NA (HEC-RAS) | 1 |
| High Ridge | 2-8 x 4 ft Box | 79 | 19 | 705 | Franklin Hills St | A1A,HR3 | 10 |
| High Ridge | 2-8 x 4 ft Box | 77 | 20 | 776 | Franklin Crest Dr | A1A,HR3 | 20 |
| High Ridge | 48 ft wide Bridge | 85 | 23 | >3950 | Redd Rd | HR1,LDE1 | >100 |
| High Ridge | 67 ft wide Bridge | 120 | 16 | >3950 | Resler Dr | HR1,LDE1 | >100 |
| Montoya Drain | 1-18-inch Circular | 64 | 9 | 16 | Montoya Dr | MD_3 | 1 |
| Montoya Drain | 50 ft wide Bridge | 72 | 4 | >1049 | Redd Rd | MD_3 | >100 |
| Montoya Drain | 1-24-inch Circular | 178 | 8 | 23 | Railroad | MD_3 | 1 |
| Montoya Drain | 1-36-inch Circular | 75 | 11 | 74 | Mulberry Ave | MD_3 | 2 |
| Montoya Drain | 1-36-inch Circular | 59 | 11 | 76 | Lindbergh St | MD_3 | 3 |
| Montoya Drain | no existing information | NA | NA | NA | Country Club 2 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Country club 1 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Golf 2 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Golf 1 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Golfcart Crossing 5 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Golfcart Crossing 4 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Golfcart Crossing 3 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Golfcart Crossing 2 | MD_3 | NA |
| Montoya Drain | no existing information | NA | NA | NA | Golfcart Crossing 1 | MD_3 | NA |
| Montoya Drain | 1-48-inch Circular | 88 | 12 | 144 | Lombardy Ave | MD2,C2 | 5 |
| Montoya Drain | 1-48-inch Circular | 73 | 12 | 145 | Sunset Dr | MD2,C2 | 5 |
| Montoya Drain | 64 ft wide Bridge | 50 | 8 | 1040 | Turnstone Dr | MD2,WSD1 | 20 |

Table B-12. Culvert Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|--------------------|-------------|-----------------|------------------------|---------------------|-------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Montoya Drain | 59 ft wide Bridge | 5 | 7 | 1490 | Pedestrian Overpass | MD2,WSD1 | 40 |
| Montoya Drain | 45 ft wide Bridge | 32 | 9 | 1150 | Frontera Rd | MD2,WSD1 | 25 |
| Montoya Drain | 1-72-inch Circular | 140 | 10 | 160 | Sunland Park Rd | MD2,WSD1 | 2 |
| Montoya Drain | 1-96-inch Circular | 74 | 10 | 235 | Racetrack 2 | MD2,WSD1 | 3 |
| Montoya Drain | 60 ft wide Bridge | 17 | 7 | >2155 | Racetrack 1 | MD2,WSD1 | >100 |
| Montoya Drain | 57 ft wide Bridge | 44 | 7 | >2155 | Dona Ana County Rd | MD2,WSD1 | >100 |
| Montoya Drain | 3-5 x 5 ft Box | 45 | 11 | 660 | Unknown | MD2,WSD1 | 10 |
| Ojo de Agua | 1-16 x 5 ft Box | 102 | 20 | >785 | Via de la Paz Dr | ODA2 | >100 |
| Ojo de Agua | 2-6 x 4 ft Box | 98 | 24 | 657 | Via Descanso Ln | ODA2 | 60 |
| Ojo de Agua | 1-9 x 9 ft Box | 88 | 29 | 1287 | Loma de Cristo Dr | ER1,ODA1 | 20 |
| Ojo de Agua | 1-9 x 8 ft Box | 161 | 22 | 1021 | Westwind Dr | ER1,ODA1 | 10 |
| Ojo de Agua | 1-30 x 14 ft Arch | 190 | 26 | >3412 | Resler Dr | ER1,ODA1 | >100 |
| Ojo de Agua | 49 ft wide bridge | 46 | 17 | >3412 | Lakehurst Rd | ER1,ODA1 | >100 |
| Resler Channel | 58 ft wide Bridge | 12 | 14 | >3025 | Pedestrian Overpass | RC2cUS | >100 |
| Resler Channel | 59 ft wide Bridge | 121 | 17 | >3025 | Resler Dr | RC2cUS | >100 |
| Resler Channel | 4-10 x 8 ft Box | 105 | 21 | >3025 | Northwestern Dr | RC2cUS | >100 |
| Resler Channel | 112 ft wide Box | 140 | 16 | >3025 | IH-10 | RC2cUS | >100 |
| Ridge View | 2-9 x 5 ft Box | 119 | 16 | >1285 | Franklin Hills St | RV_1 | >100 |
| Ridge View | 2-8 x 6 ft Box | 82 | 21 | 960 | Shelby Ridge | RV_1 | 40 |
| Ridge View | 2-6 x 4 ft Box | 110 | 25 | 880 | Desert Canyon Dr | RV_1 | 40 |
| Unknown 2 | 3-48-inch Circular | 360 | 15 | >395 | IH-10 | UN02_1OUT | >100 |
| Unnamed Stream 23A | 1-4 x 4 ft Box | 340 | 16 | >71 | IH-10 | UN23A | >100 |
| Unnamed Stream 24 | 2-48-inch Circular | 369 | 17 | >133 | IH-10 | UN24A2 | >100 |

Table B-12. Culvert Capacity Summary - Northwest Region (Continued)

| NORTHWEST REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|---|-------------------------|-------------|-----------------|------------------------|------------------|---------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Unnamed Stream 24A | 3-60-inch Circular | 385 | 13 | >385 | IH-10 | UN24A1OUT | >100 |
| Unnamed Stream 24B | 2-60-inch Circular | 340 | 16 | >290 | IH-10 | UN24B | >100 |
| Unnamed Stream 24C | no existing information | NA | NA | 138.3 | IH-10 | UN24C | NA |
| Unnamed Stream 24D | no existing information | NA | NA | 54.4 | IH-10 | UN24D | NA |
| White Spur Drain | 2-7 x 4 ft Box | 76 | 15 | >620 | Doniphan Dr | WSD_2 | >100 |
| White Spur Drain | 50 ft wide Bridge | 9 | 7 | 960 | Railroad | DD3,WSD2 | 80 |
| White Spur Drain | 30 ft wide Bridge | 31 | 5 | >1029 | Love Rd | DD3,WSD2 | >100 |
| White Spur Drain | 35 ft wide Bridge | 53 | 12 | >907 | Sunset Dr | WSD_1, WSD1_1 | >100 |
| White Spur Drain | 3-48-inch Circular | 66 | 9 | 345 | River Bend Dr | WSD_1, WSD1_1 | 8 |

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Table B-13. Culvert Capacity Summary - West Central Region

| WEST CENTRAL REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|--|-------------------------|-------------|-----------------|------------------------|------------------|-------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Canterbury Channel | 2-10 x 5.5 ft Arch | 72 | 10 | >605 | Stanton St | CBC_1 | >100 |
| Canterbury Channel | 2-10 x 5.5 ft Arch | 51 | 11 | >666 | Ridgecrest Dr | CBC_1 | >100 |
| Canterbury Channel | 5-4 x 4 ft Box | 138 | 11 | >757 | Mesa St | CBC_1 | >100 |
| Canterbury Channel | 2-6 x 6 ft Box | 538 | 24 | >757 | Wallington Dr | CBC_1 | >100 |
| Canterbury Channel | 3-6 x 6 ft Box | 401 | 14 | >1009 | IH-10 | CBC_1 | >100 |
| Flow Path No. 20 | 1-72-inch Circular | 82 | 18 | 345 | Zenith Dr | FPN20_2 | 6 |
| Flow Path No. 20 | 3-4 x 4 ft Box | 2040 | 28 | 375 | Mesa St | FPN20_2 | 4 |
| Flow Path No. 20 | no existing information | NA | NA | NA | Mesa Park | FPN20_2 | NA |
| Flow Path No. 20 | 1-228-inch Circular | 398 | 17 | >2949 | IH-10 | FPN20_2 | >100 |
| Flow Path No. 20 | no existing information | NA | NA | NA | Unknown | FPN20_1US | NA |
| Flow Path No. 20 | no existing information | NA | NA | NA | Factory1 | FPN20_1US | NA |
| Flow Path No. 20 | no existing information | NA | NA | NA | Railroad 1 | FPN20_1US | NA |
| Flow Path No. 20 | no existing information | NA | NA | NA | Railroad 2 | FPN20_1US | NA |
| Flow Path No. 20 | 5-8 x 3 ft Box | 186 | 17 | 1050 | Paisano Dr | FPN20_1US | 4 |
| Flow Path No. 21 | 1-98 x 63-inch Ellipse | 60 | 22 | 430 | Okeefe Dr | FPN21_2 | 5 |
| Flow Path No. 21 | 2-4 x 4 ft Box | 770 | 18 | 579 | Mesa St | FPN21_2 | 7 |
| Flow Path No. 21 | 1-15.5 x 10 ft Box | 470 | 41 | >2293 | IH-10 | FPN21_2 | >100 |
| Flow Path No. 21 | no existing information | NA | NA | NA | Railroad 3 | FPN21_2 | NA |

Table B-13. Culvert Capacity Summary - West Central Region (Continued)

| WEST CENTRAL REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|--|--------------------------|-------------|-----------------|------------------------|------------------------|-------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 21 | no existing information | NA | NA | NA | Railroad 2 | FPN21_2 | NA |
| Flow Path No. 21 | no existing information | NA | NA | NA | Railroad 1 | FPN21_OUT | NA |
| Flow Path No. 21 | 2-6 x 5 ft Box | 150 | 9 | 500 | Paisano Dr | FPN21_OUT | 4 |
| Flow Path No. 23 | no existing information | NA | NA | NA | Robinson Ave | FPN23_1 | NA |
| Flow Path No. 23 | 2-72-inch Circular | 71 | 14 | 610 | Campbell St | FPN23_1 | 4 |
| Flow Path No. 23 | 2-10 x 5 ft Box | 52 | 15 | 700 | Kansas St | FPN23_1 | 5 |
| Flow Path No. 23 | 2-12.5 x 5.5 ft Box | 400 | 24 | 1250 | Mesa St and Stanton St | FPN23_1 | 9 |
| Flow Path No. 23 | 2-84-inch Circular | 71 | 17 | 1200 | Oregon St | FPN23_1 | 8 |
| Flow Path No. 23 | 1-141.8 x 91.3-inch Arch | 86 | 29 | 1460 | University Ave | FPN23_1 | 10 |
| Flow Path No. 23 | no existing information | NA | NA | NA | FPN23 CV Building 3 | FPN23_1 | NA |
| Flow Path No. 23 | no existing information | NA | NA | NA | FPN23 CV Hawthorn | FPN23_1 | NA |
| Flow Path No. 23 | no existing information | NA | NA | NA | FPN23 CV Building 2 | FPN23_1 | NA |
| Flow Path No. 23 | no existing information | NA | NA | NA | FPN23 CV Walkway | FPN23_1 | NA |
| Flow Path No. 23 | no existing information | NA | NA | NA | FPN23 CV Building 1 | FPN23_1 | NA |
| Flow Path No. 23 | 1-168-inch Circular | 1766 | 26 | >2987 | IH-10 and Schuster Ave | FPN23_1 | >100 |
| Flow Path No. 23 | no existing information | NA | NA | NA | Railroad | FPN23_1 | NA |

Table B-13. Culvert Capacity Summary - West Central Region (Continued)

| WEST CENTRAL REGION - CULVERT CAPACITY SUMMARY | | | | | | | |
|--|----------------|-------------|-----------------|------------------------|------------------|-------------|--------------------------------------|
| Channel | Shape/Size | Length (ft) | Velocity (ft/s) | Culvert Capacity (cfs) | Culvert Location | HMS Node ID | Interpolated Return Interval (years) |
| Flow Path No. 23 | 6-8 x 3 ft Box | 42 | 12 | 1100 | US of Paisano Dr | FPN23_1 | 8 |
| Flow Path No. 23 | 4-7 x 7 ft Box | 182 | 20 | 2700 | Paisano Dr | FPN23_1 | 50 |
| Flow Path No. 23 | 4-6 x 4 ft Box | 43 | 13 | 940 | DS of Paisano Dr | FPN23_1 | 7 |
| Paragon Channel | 2-6 x 4 Box | 252 | 24 | >595 | Stanton St | PC_1 | >100 |
| Paragon Channel | 5-6 x 4 ft Box | 303 | 24 | >1488 | Mesa St | PC_1 | >100 |
| Paragon Channel | 5-8 x 8 ft Box | 309 | 17 | >2975 | IH-10 | PC_1 | >100 |

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Table B-14. Conduit Capacity Summary - Central Region

| CENTRAL REGION - CONDUIT CAPACITY SUMMARY | | | | | | | |
|---|------------------------------------|------|------------------|-------------|-------------|----------------|---|
| System | Name | Type | Dimensions | Length (ft) | Pressurized | Capacity (cfs) | Comment |
| Government Hills Channel | Government Hills Discharge 90-inch | Pipe | 90-Inch Diameter | 9308 | Yes | 350 | |
| Cebada | Cebada Discharge Conduits | Box | 2 (6x 4) | 5452 | No | 540 | Capacity is based on Brock & Bustillos, Inc. Report |
| Dallas | Eastern Discharge Conduit | Box | 6 x 5 | 2395 | No | 360 | Capacity is maximum capacity from MCI Report |
| Dallas | Western Discharge Conduit | Box | 7 x 5 | 4964 | No | 441 | Capacity is maximum capacity from MCI Report |

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FIGURES

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Legend

- HECRAS-Modeled Stream
- Master Plan Regional Area
- Drainage System
- Major Road
- Railroad
- State Boundary
- County Boundary
- Corporate Boundary
- Military Boundary
- Park Boundary

Elevation:

- 7,001 - 7,180
- 6,751 - 7,000
- 6,501 - 6,750
- 6,251 - 6,500
- 6,001 - 6,250
- 5,751 - 6,000
- 5,501 - 5,750
- 5,251 - 5,500
- 5,001 - 5,250
- 4,751 - 5,000
- 4,501 - 4,750
- 4,251 - 4,500
- 4,001 - 4,250
- 3,751 - 4,000
- 3,517 - 3,750



0 1.5 3 Miles
1 : 190,080 or 1 inch = 3 miles

Project Area

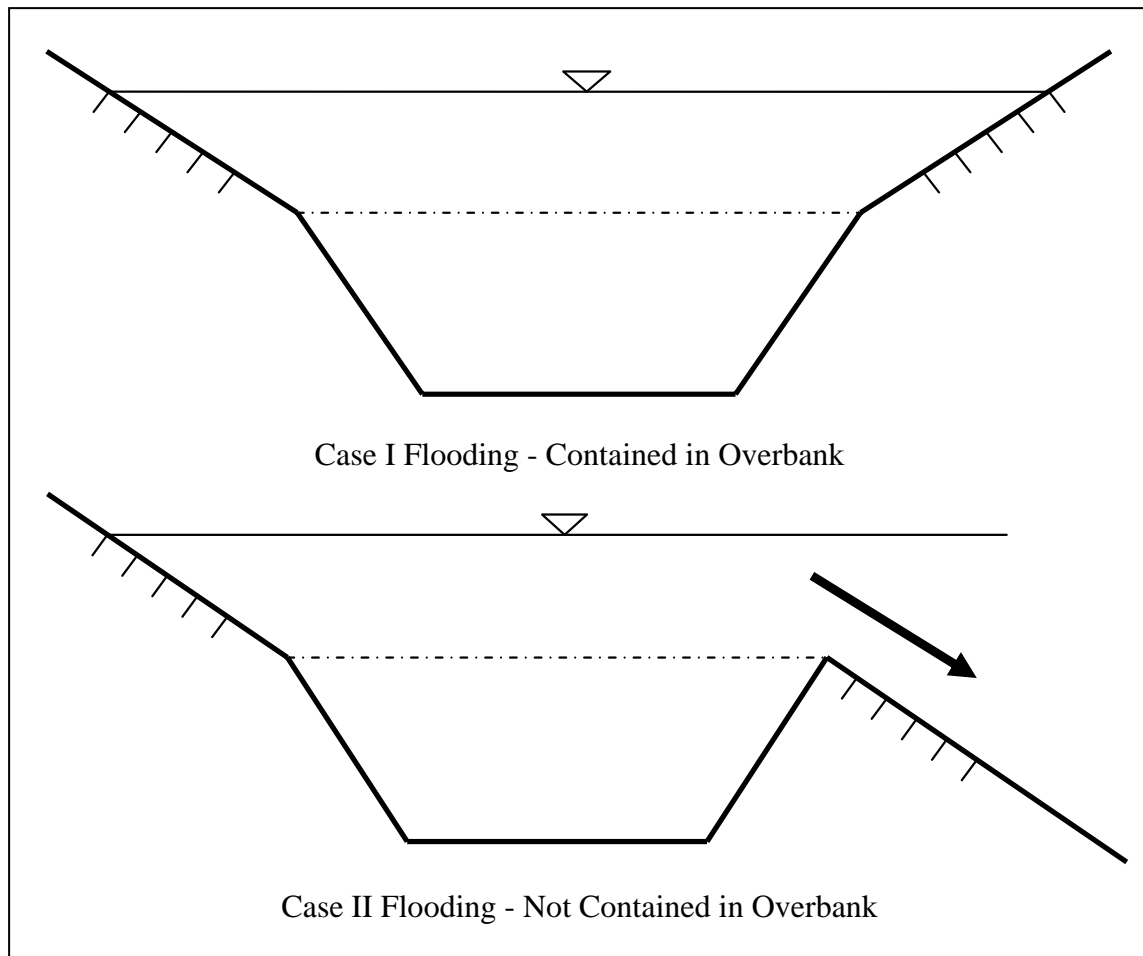
El Paso Stormwater Master Plan

Date: 03/05/2009

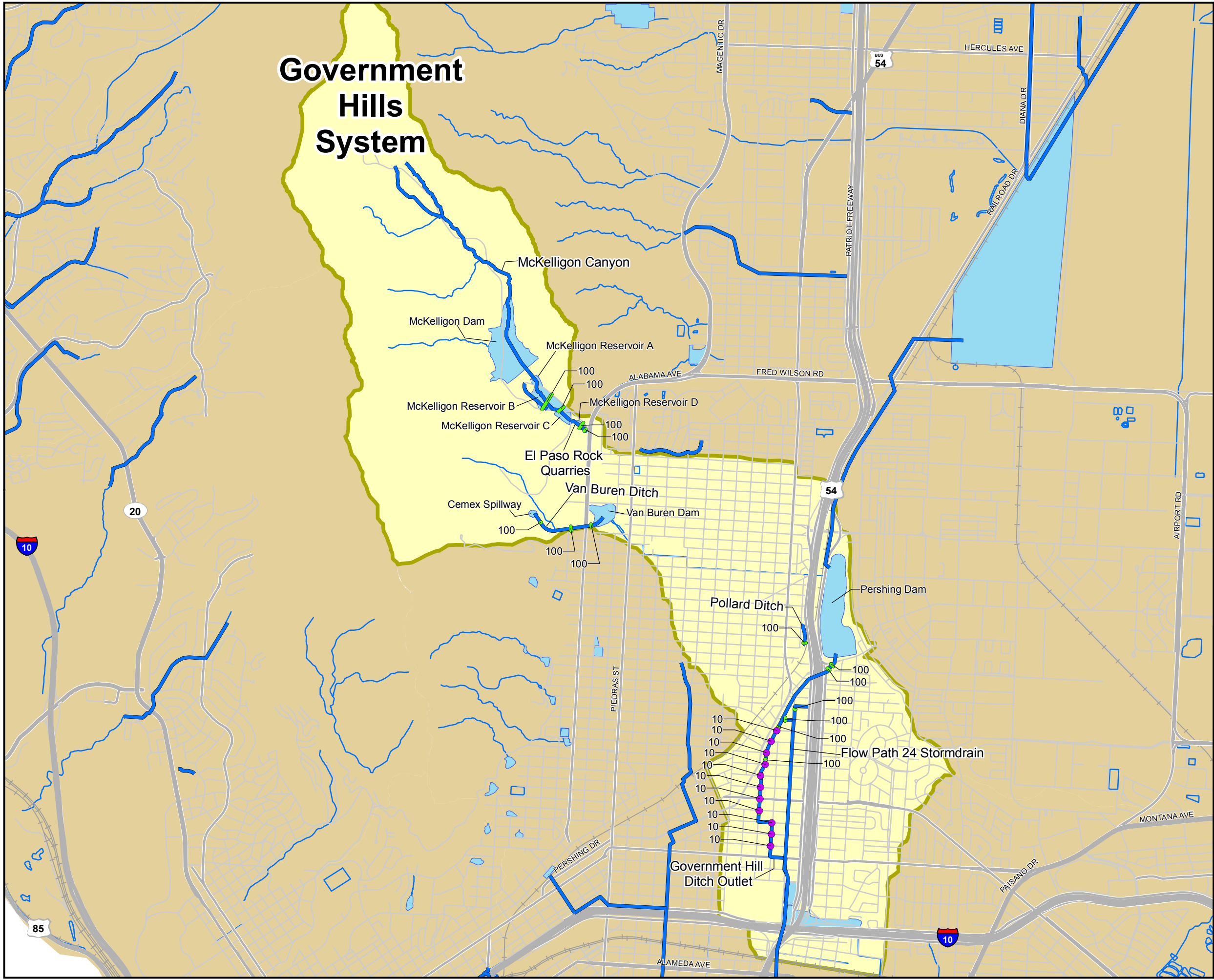
Figure B-1



Figure B-2 Typical Cross-Sections - Case I and Case II



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Legend

- Crossing Capacity
- Channel Capacity
- Study Reach
- Streamline
- Major Road
- Minor Road
- Railroad
- Dam/Basin
- Goverment Hills System
- Other Drainage Systems



0 1,500 3,000
Feet
1 : 36,000 or 1 Inch = 3,000 Feet

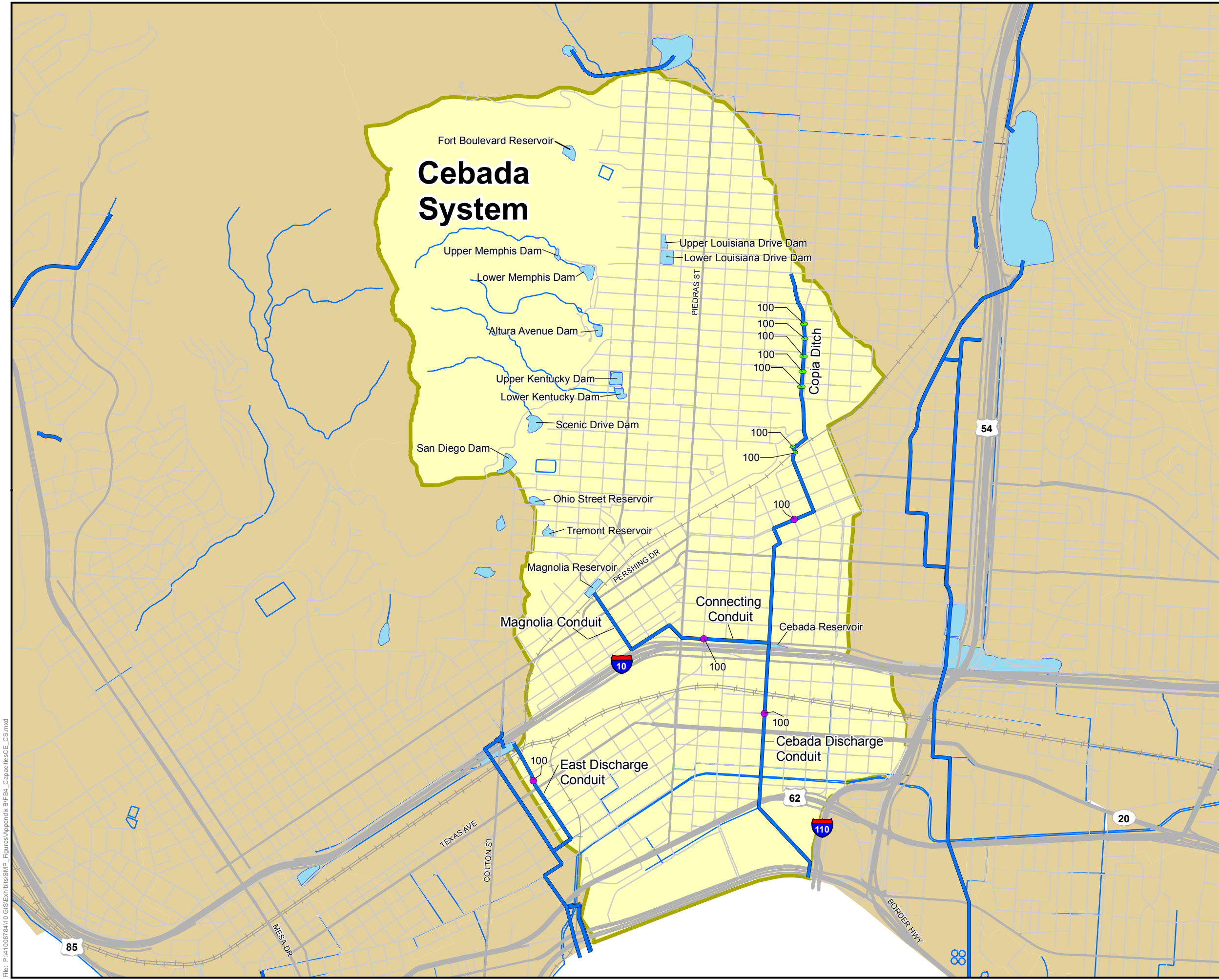
**Government Hills System
Channel and Crossing Return
Period Capacities
Central Region**

El Paso Stormwater Master Plan

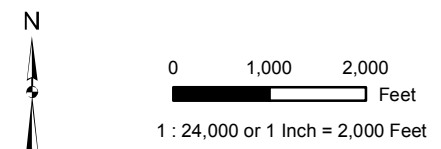
Date: 03/05/2009

Figure B-3





- Legend**
- Conduit Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Cebada System
 - Other Drainage Systems



**Cebada System
Channel and Crossing Return
Period Capacities
Central Region**

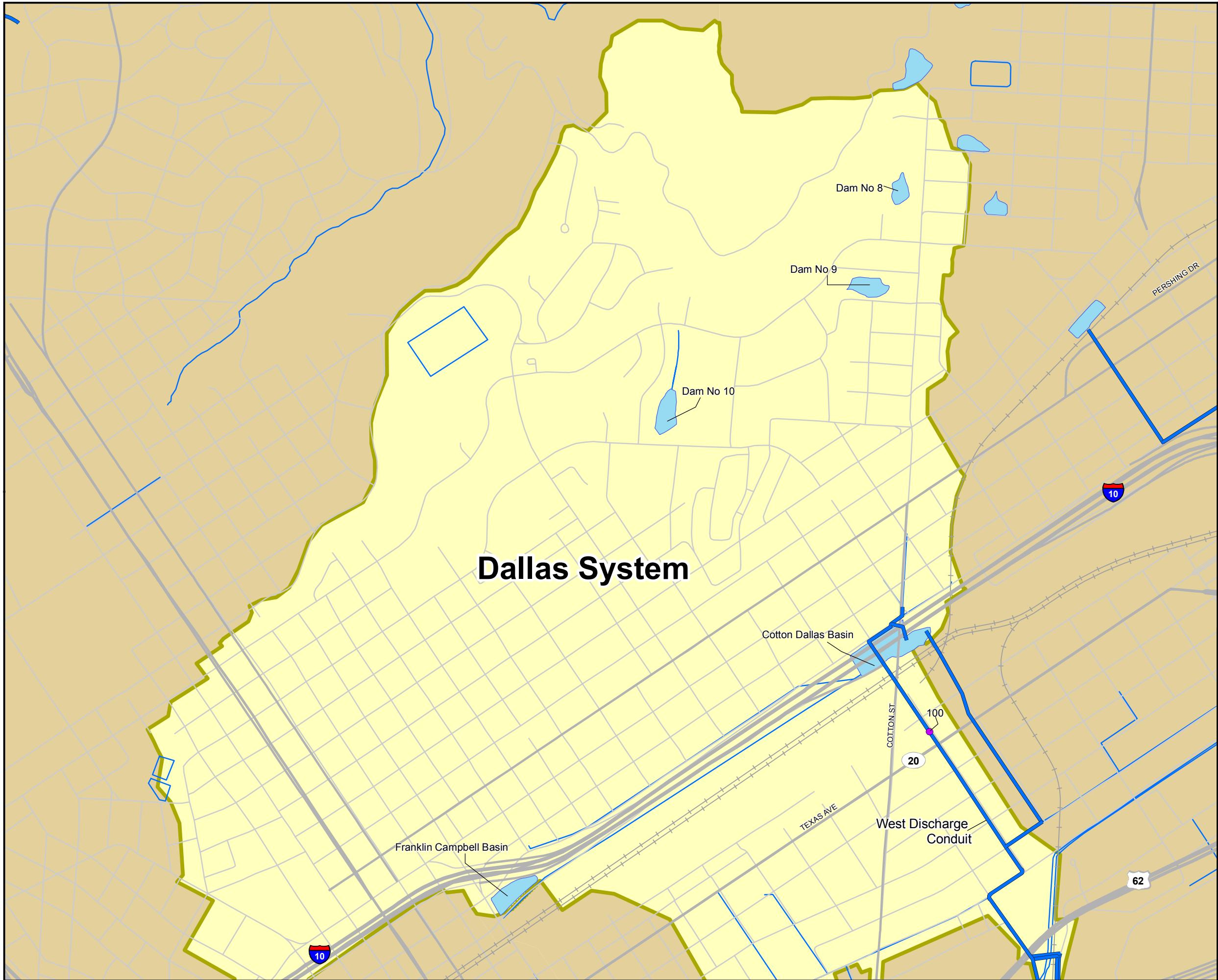
El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-4



File: P:\1008784110 GIS\Exhibits\SMP_Figures\Appendix B\FB4_CapacitiesCE_CS.mxd

File: P:\41008784\10 GIS\Exhibits\SMP_Figures\Appendix B\FB5_CapacitiesCE_DS.mxd



Legend

- Conduit Capacity
- Study Reach
- Streamline
- Major Road
- Minor Road
- Railroad
- Dam/Basin
- Dallas System
- Other Drainage Systems



0 500 1,000 Feet
1 : 12,000 or 1 Inch = 1,000 Feet

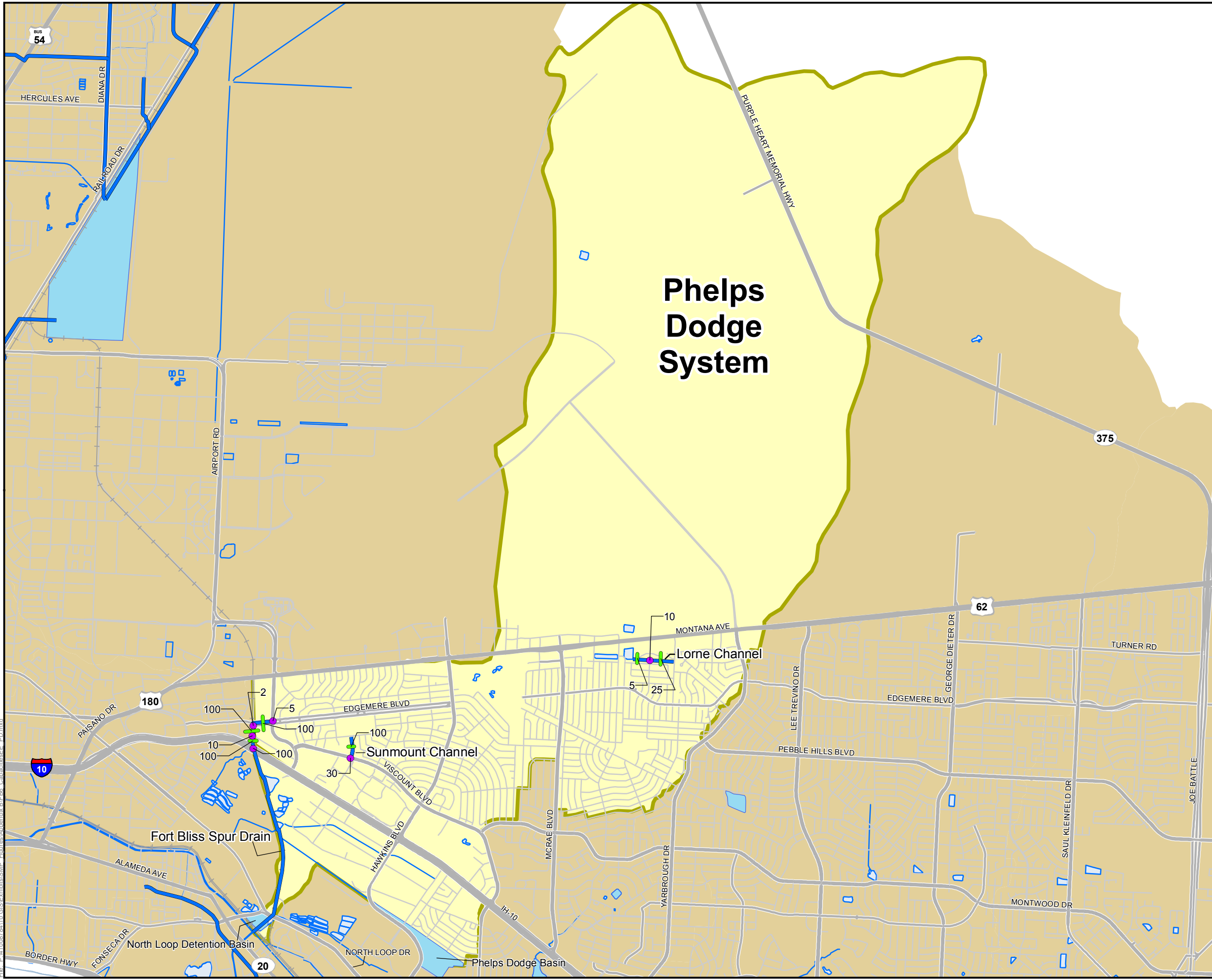
Dallas System Channel and Crossing Return Period Capacities Central Region

El Paso Stormwater Master Plan

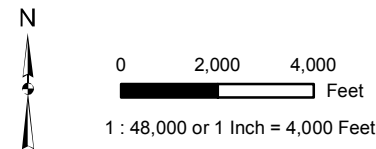
Date: 03/05/2009

Figure B-5





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - Phelps Dodge System
 - Other Drainage Systems



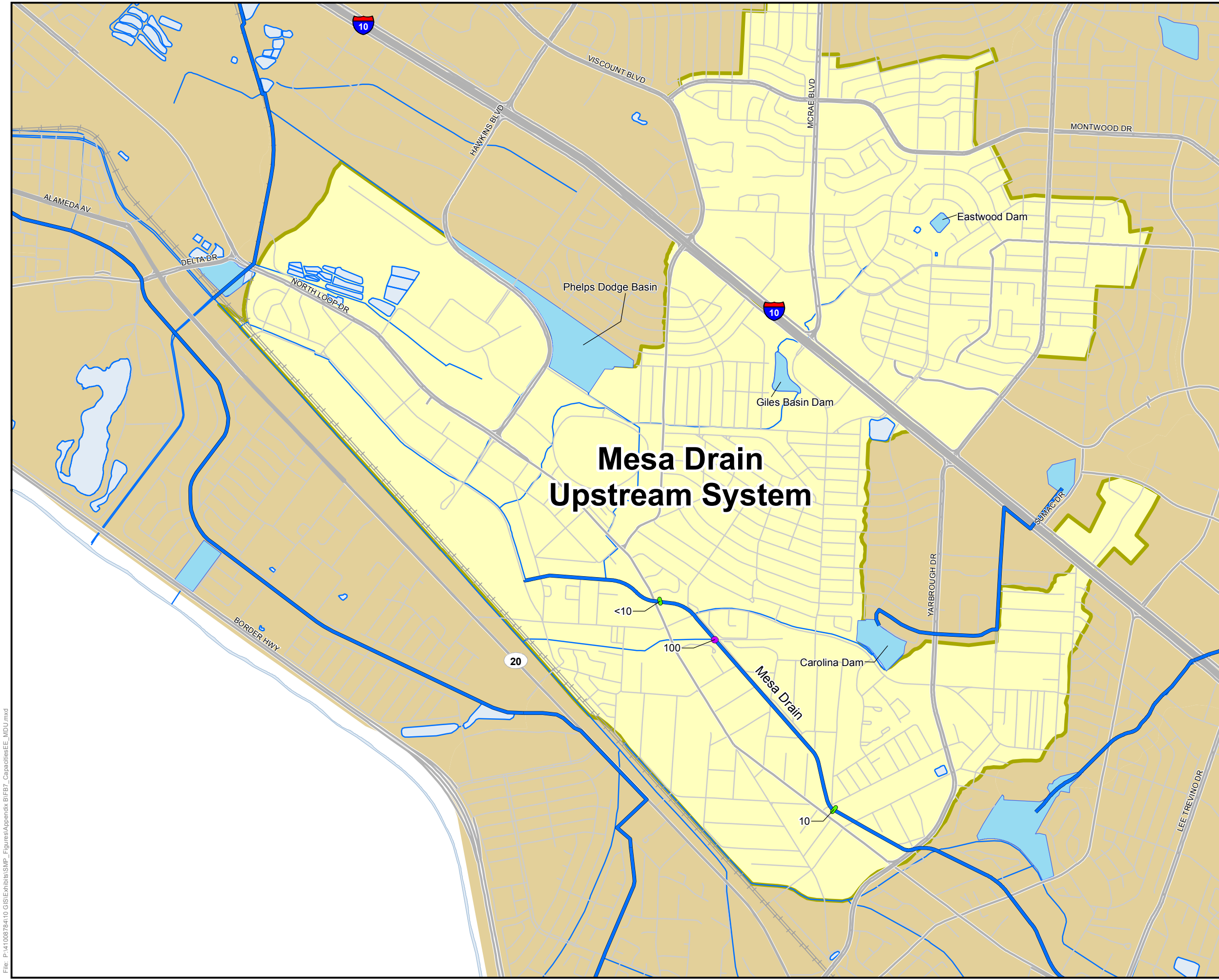
**Phelps Dodge System
Channel and Crossing Return
Period Capacities
East Side Region**

El Paso Stormwater Master Plan

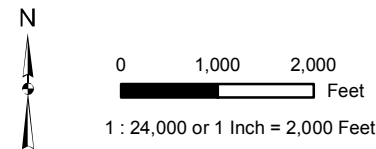
Date: 03/05/2009

Figure B-6





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - +— Railroad
 - Dam/Basin
 - Water Body
 - Mesa Drain Upstream System
 - Other Drainage Systems



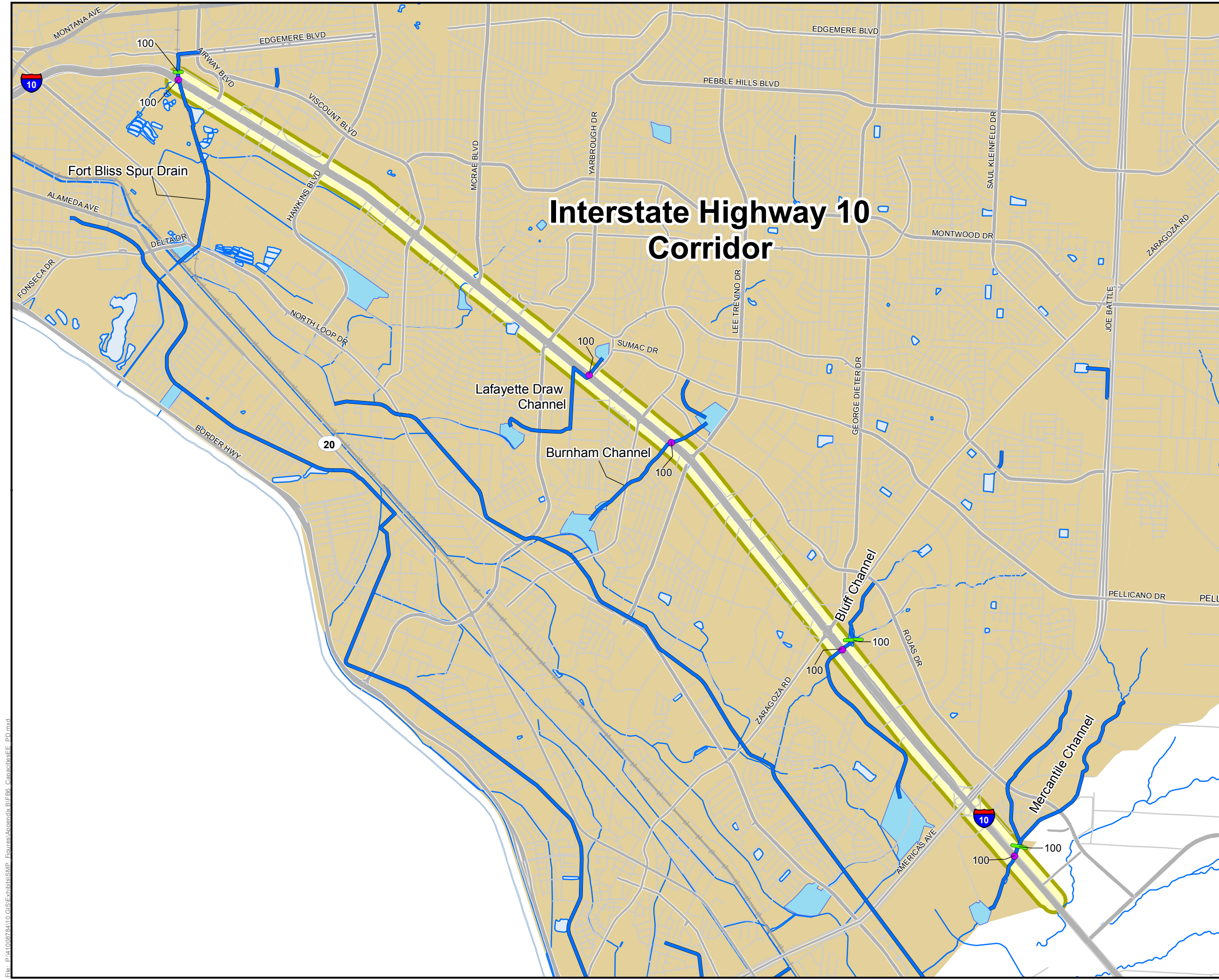
**Mesa Drain Upstream System
Channel and Crossing Return
Period Capacities
East Side Region**

El Paso Stormwater Master Plan

Date: 03/05/2009

Figure B-7





Legend

- Crossing Capacity
- Channel Capacity
- Study Reach
- Streamline
- Major Road
- Minor Road
- Railroad
- Dam/Basin
- Water Body
- Interstate Highway 10 Corridor
- Other Drainage Systems

N

0 2,000 4,000 Feet

1 : 48,000 or 1 Inch = 4,000 Feet

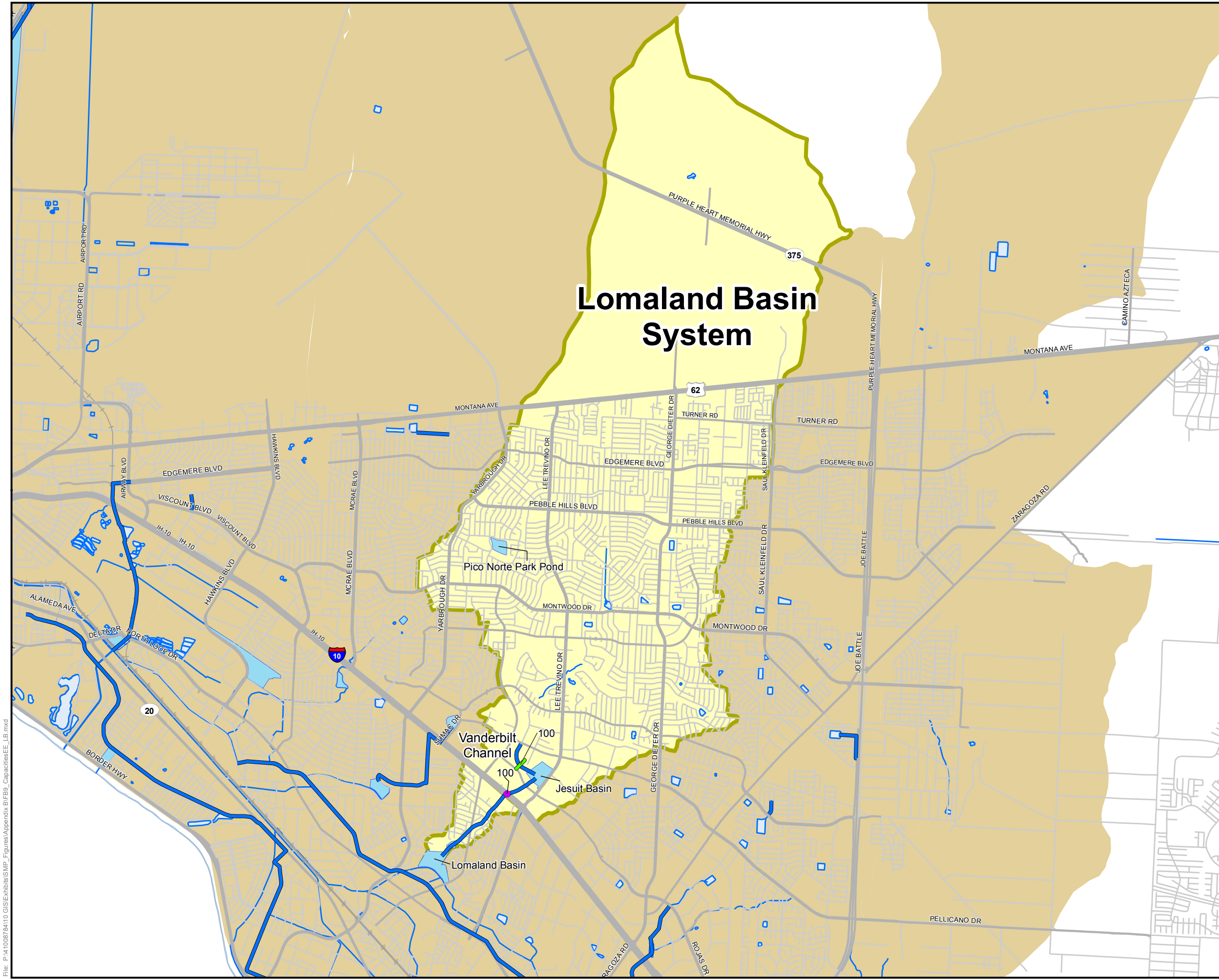
**Interstate Highway 10 Corridor
Channel and Crossing Return
Period Capacities
East Side Region**

El Paso Stormwater Master Plan

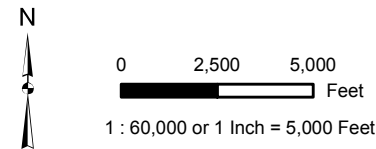
Date: 03/05/2009 Figure B-8







- Legend**
- Channel Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - Lomaland Basin System
 - Other Drainage Systems

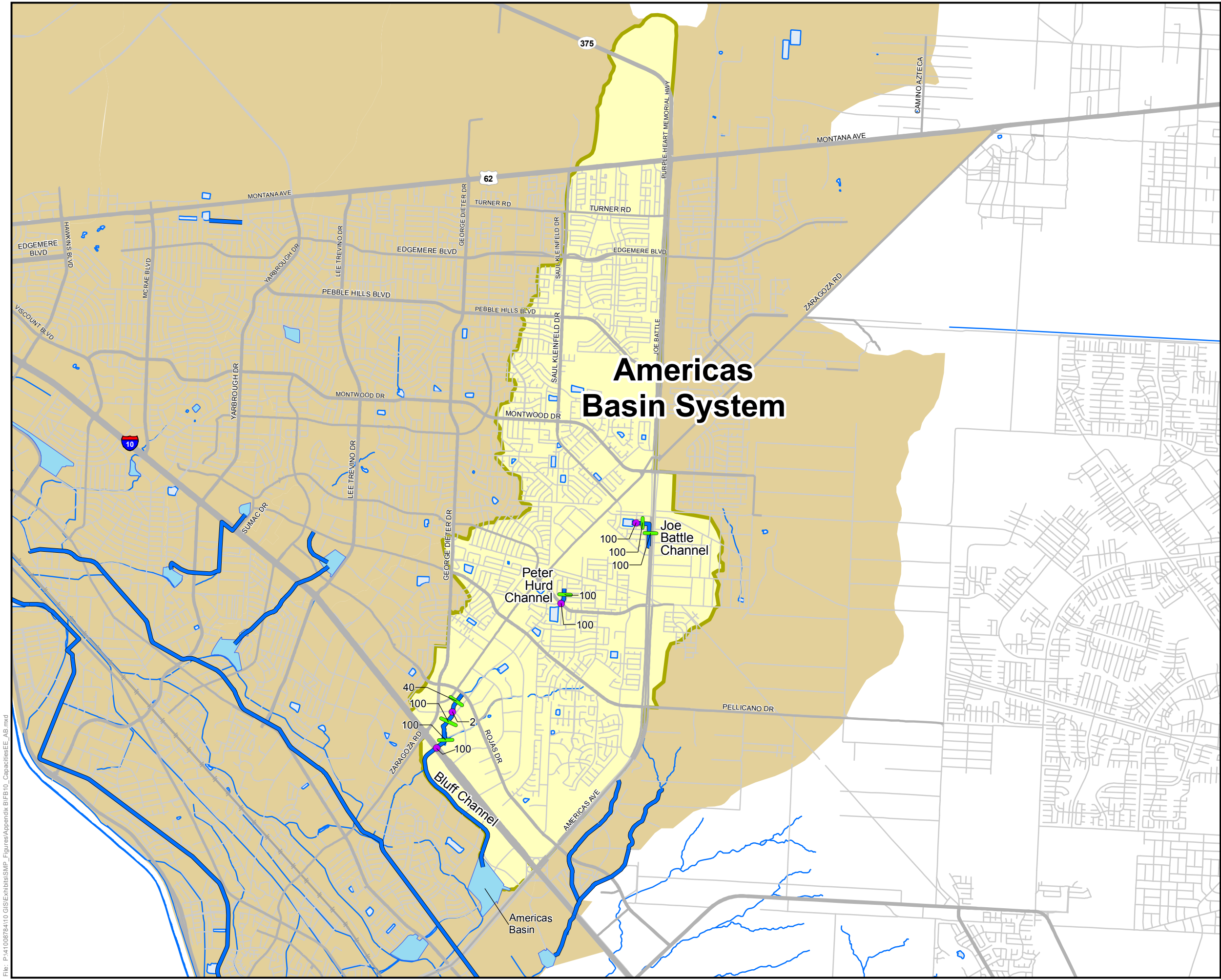


**Lomaland Basin System
Channel and Crossing Return
Period Capacities
East Side Region**

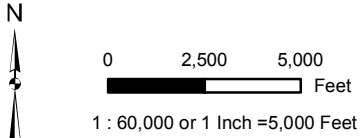
El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-9





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - Americas Basin System
 - Other Drainage Systems

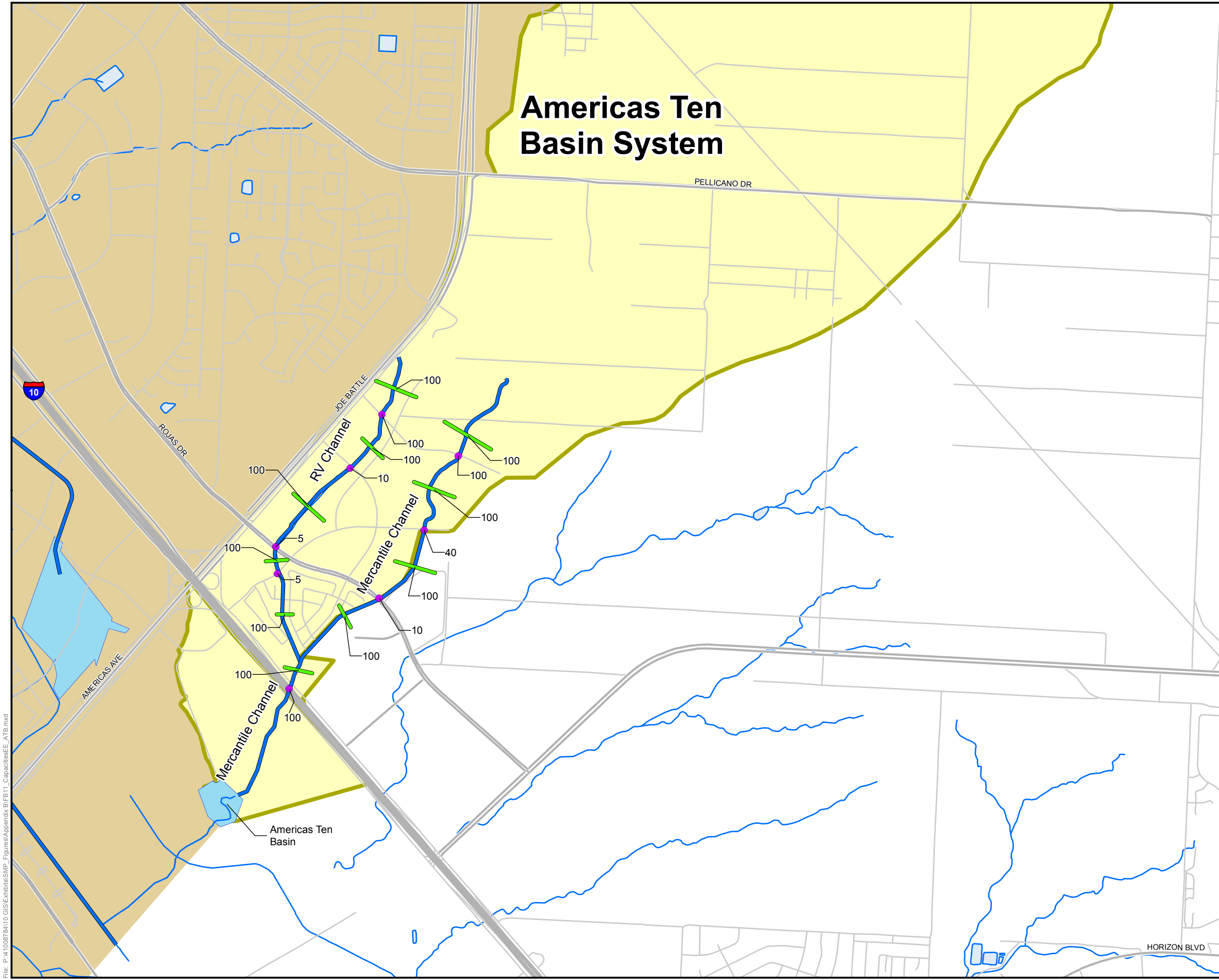


**Americas Basin System
Channel and Crossing Return
Period Capacities
East Side Region**

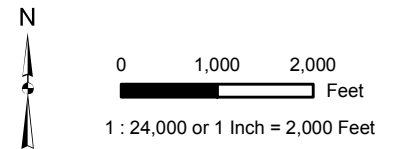
El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-10





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - + + Railroad
 - Dam/Basin
 - Water Body
 - Americas Ten Basin System
 - Other Drainage Systems



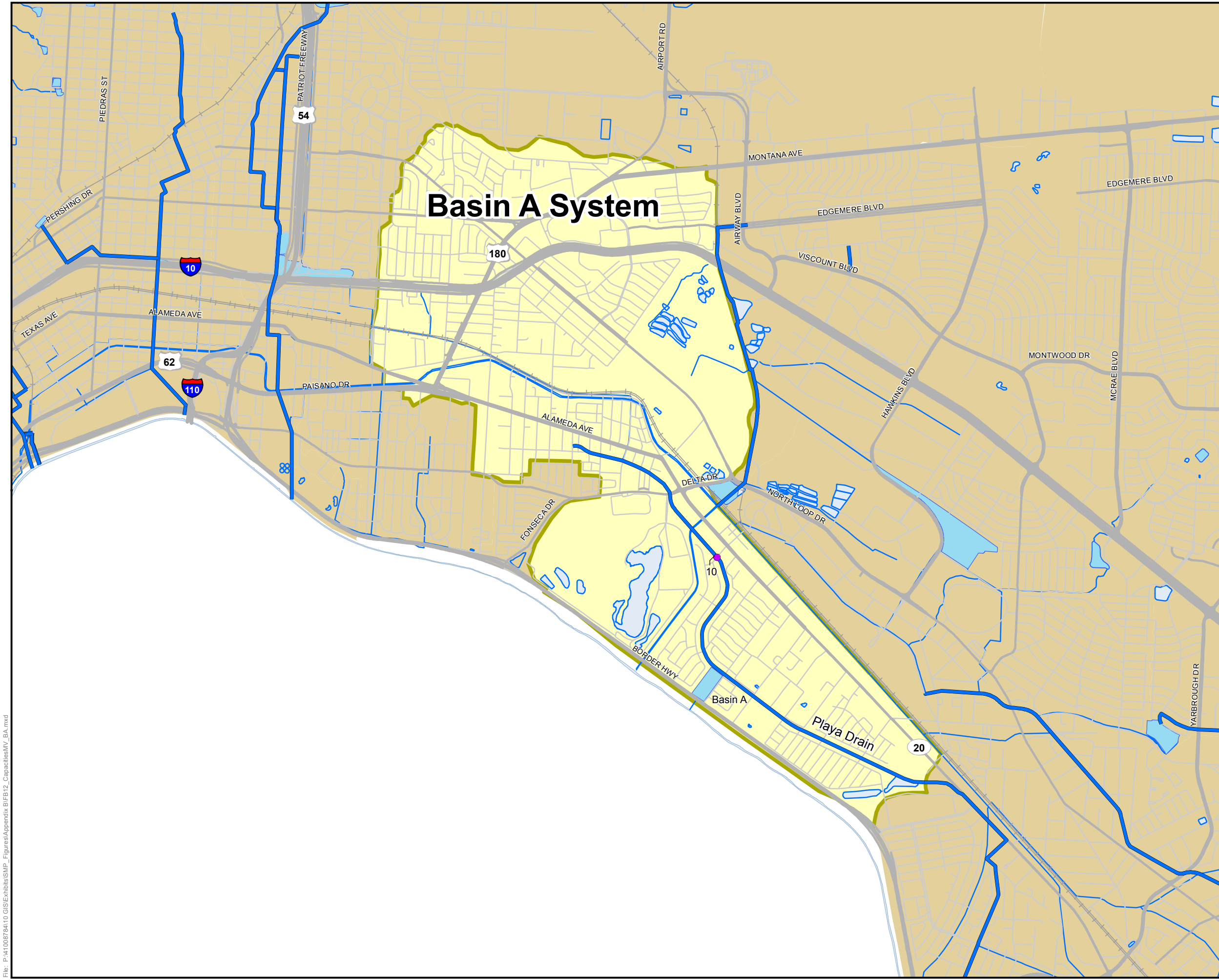
**Americas Ten Basin System
Channel and Crossing Return
Period Capacities
East Side Region**

El Paso Stormwater Master Plan

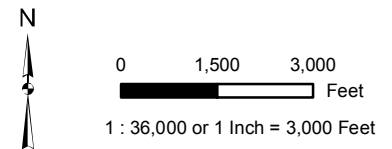
Date: 03/05/2009 Figure B-11



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- Legend**
- Crossing Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - Basin A System
 - Other Drainage Systems



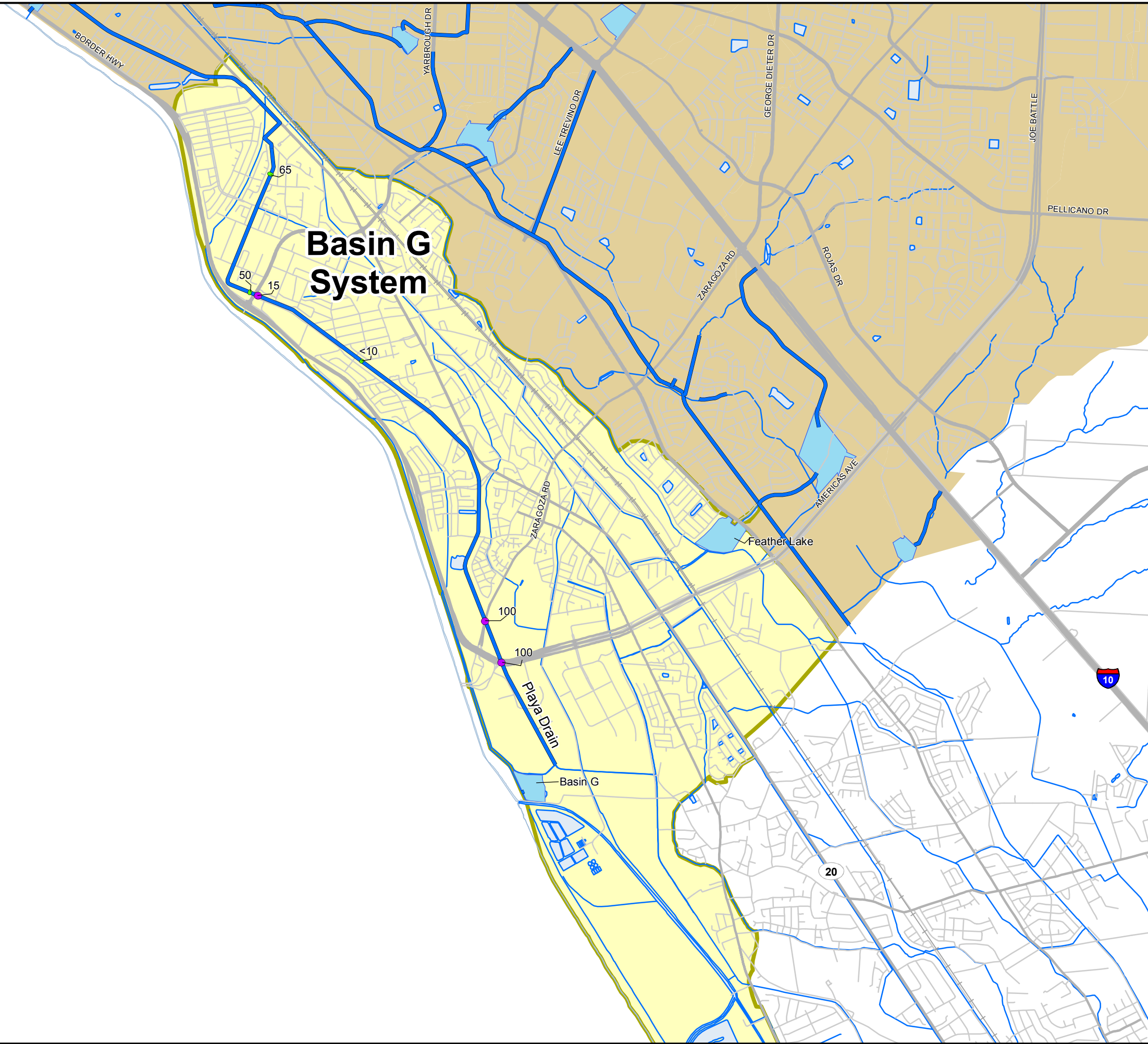
**Basin A System
Channel and Crossing Return
Period Capacities
Mission Valley Region**

El Paso Stormwater Master Plan

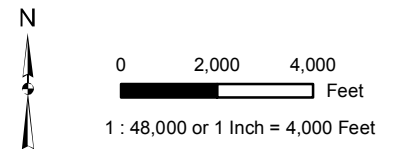
Date: 03/05/2009 Figure B-12



File: P:\4100878410 GIS\Exhibits\SMP_Figures\Appendix B\FEB13_Capacities\MV_BasinG.mxd



- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - Basin G System
 - Other Drainage Systems



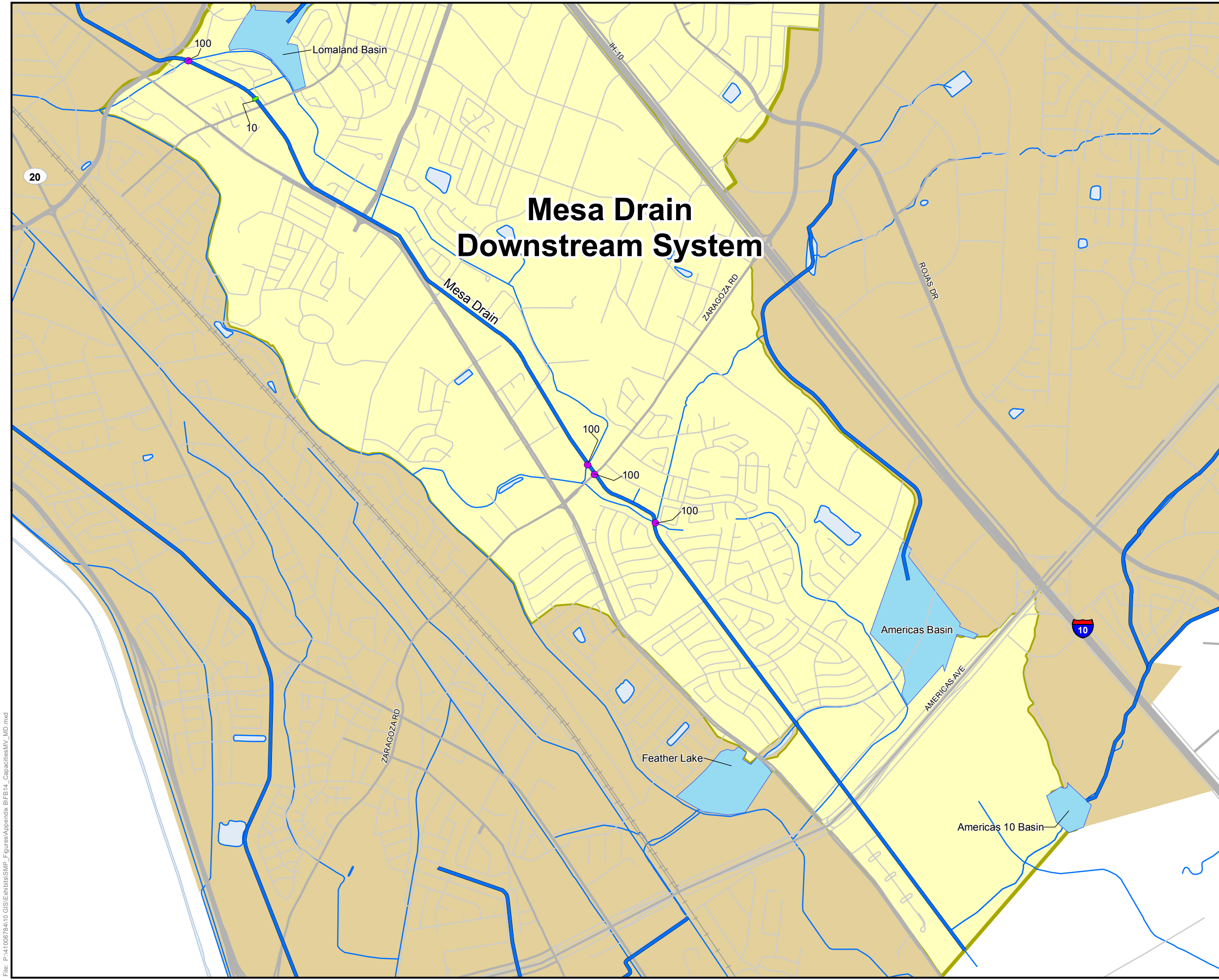
**Basin G System
Channel and Crossing Return
Period Capacities
Mission Valley Region**

El Paso Stormwater Master Plan

Date: 03/05/2009

Figure B-13





Legend

- Crossing Capacity
- Channel Capacity
- Study Reach
- Streamline
- Major Road
- Minor Road
- Railroad
- Dam/Basin
- Water Body
- Mesa Drain Downstream System
- Other Drainage Systems



0 1,000 2,000 Feet
1 : 24,000 or 1 Inch = 2,000 Feet

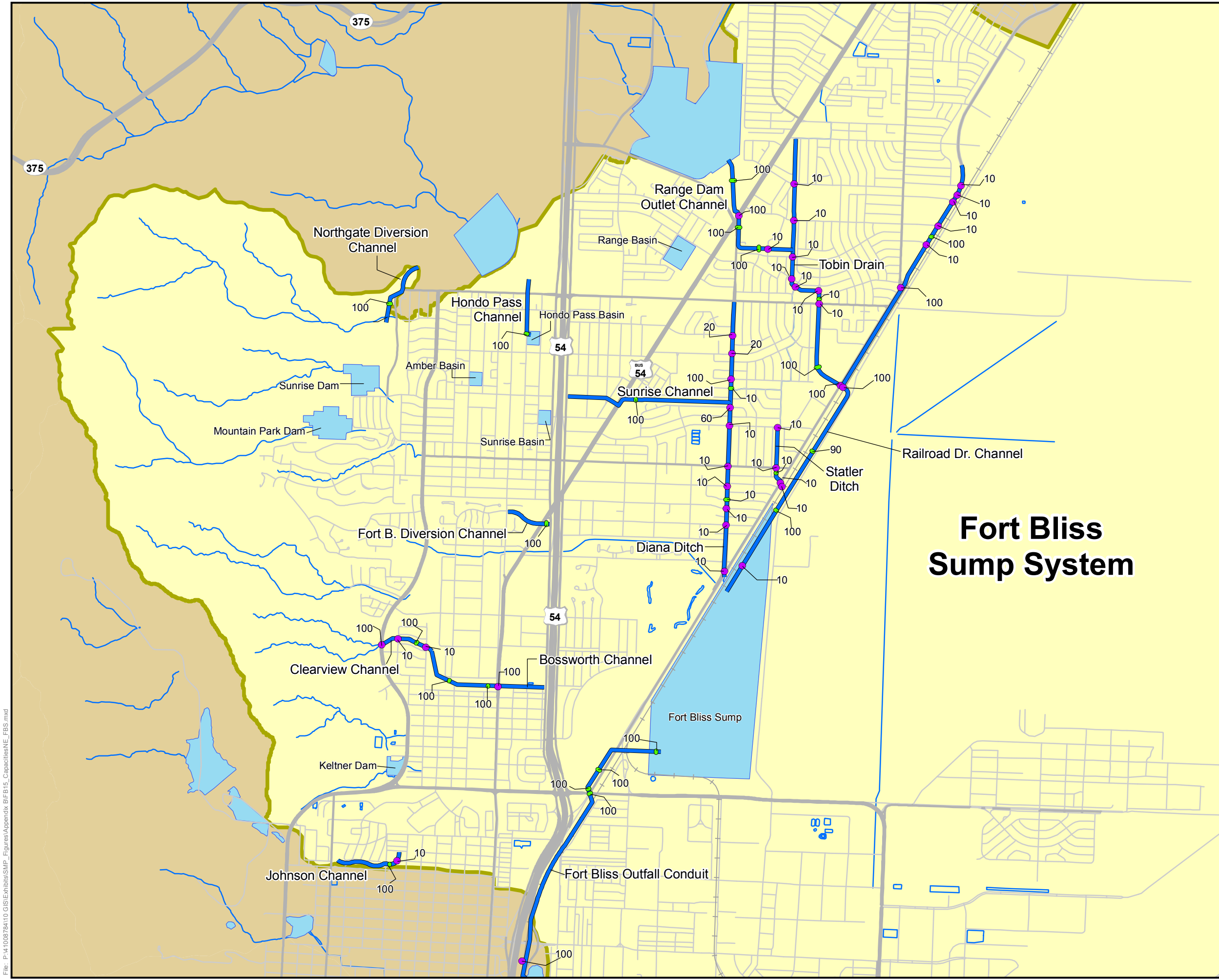
**Mesa Drain Downstream System
Channel and Crossing Return
Period Capacities
Mission Valley Region**

El Paso Stormwater Master Plan

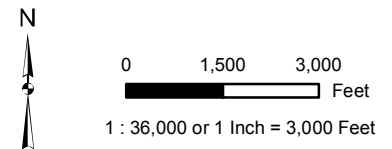
Date: 03/05/2009

Figure B-14





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Fort Bliss Sump System
 - Other Drainage Systems



**Fort Bliss Sump System
Channel and Crossing Return
Period Capacities
Northeast Region**

El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-15



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Northeast Ponding System

Legend

- Crossing Capacity
- Channel Capacity
- Study Reach
- Streamline
- Major Road
- Minor Road
- +— Railroad
- Dam/Basin
- Northeast Ponding System
- Other Drainage System



0 1,000 2,000 Feet
1 : 24,000 or 1 Inch = 2,000 Feet

Northeast Ponding System Channel and Crossing Return Period Capacities Northeast Region

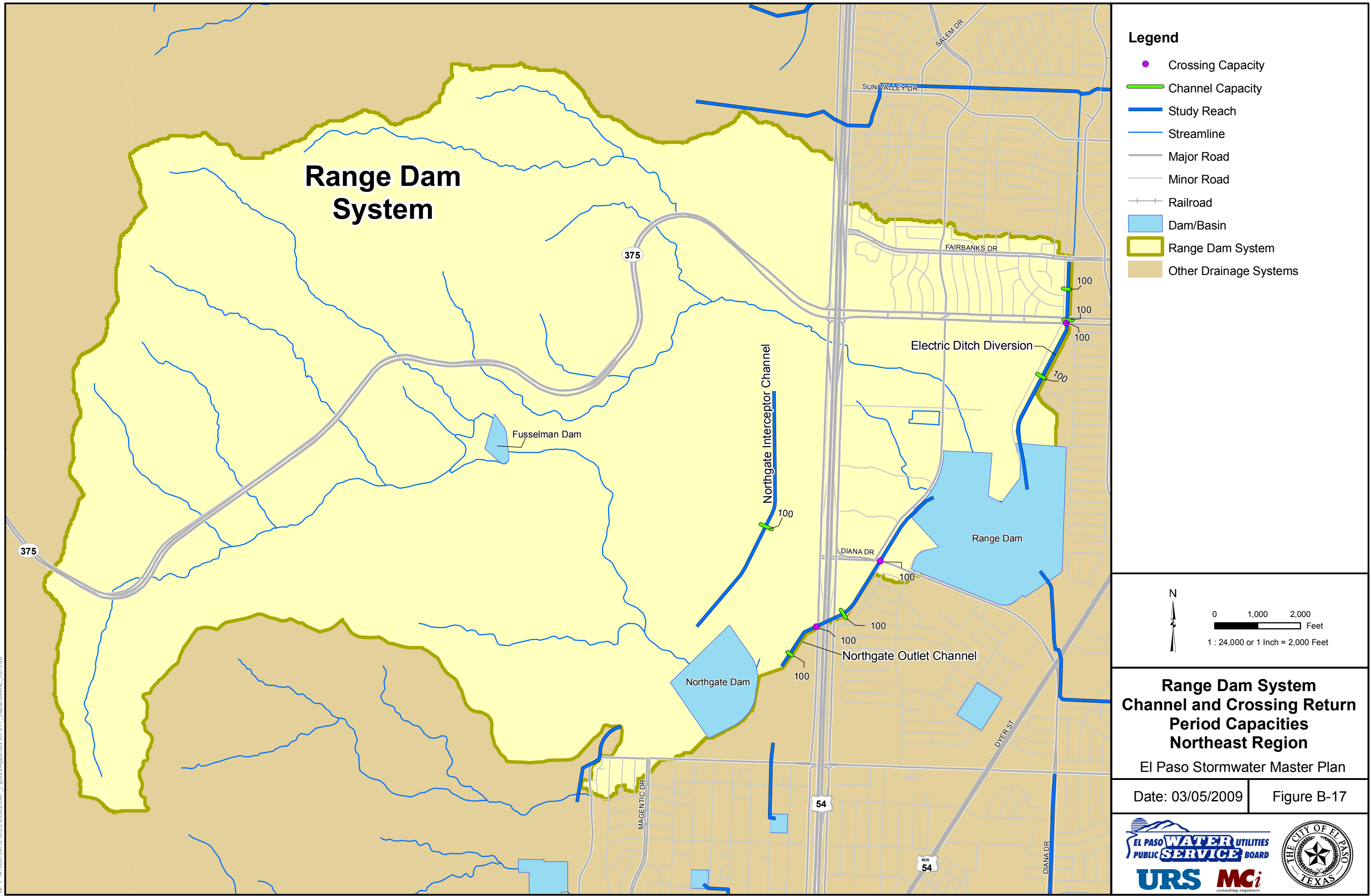
El Paso Stormwater Master Plan

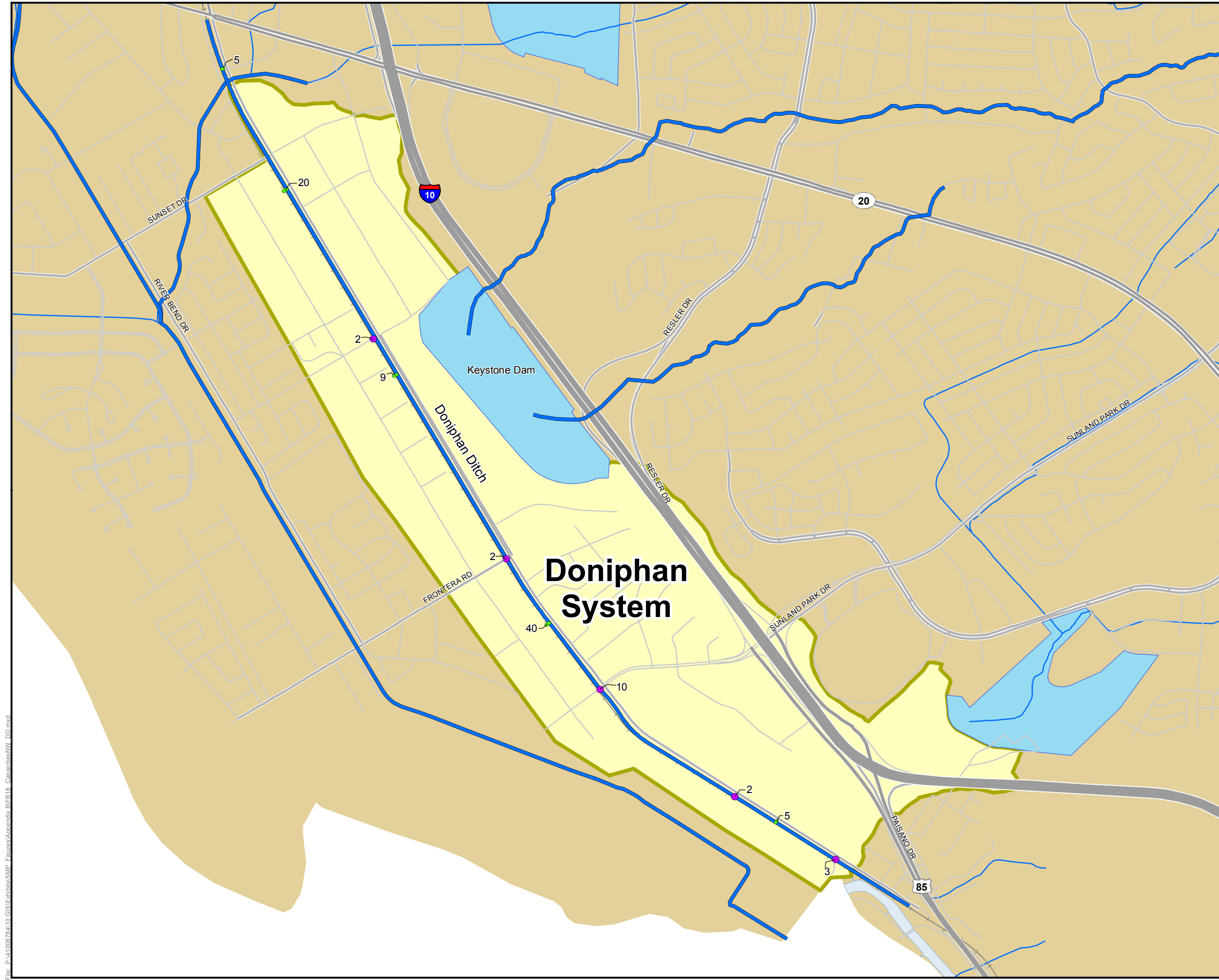
Date: 03/05/2009

Figure B-16

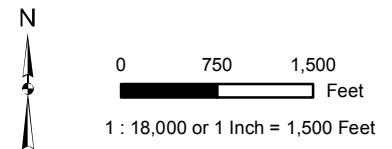


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- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - Doniphan System
 - Other Drainage Systems

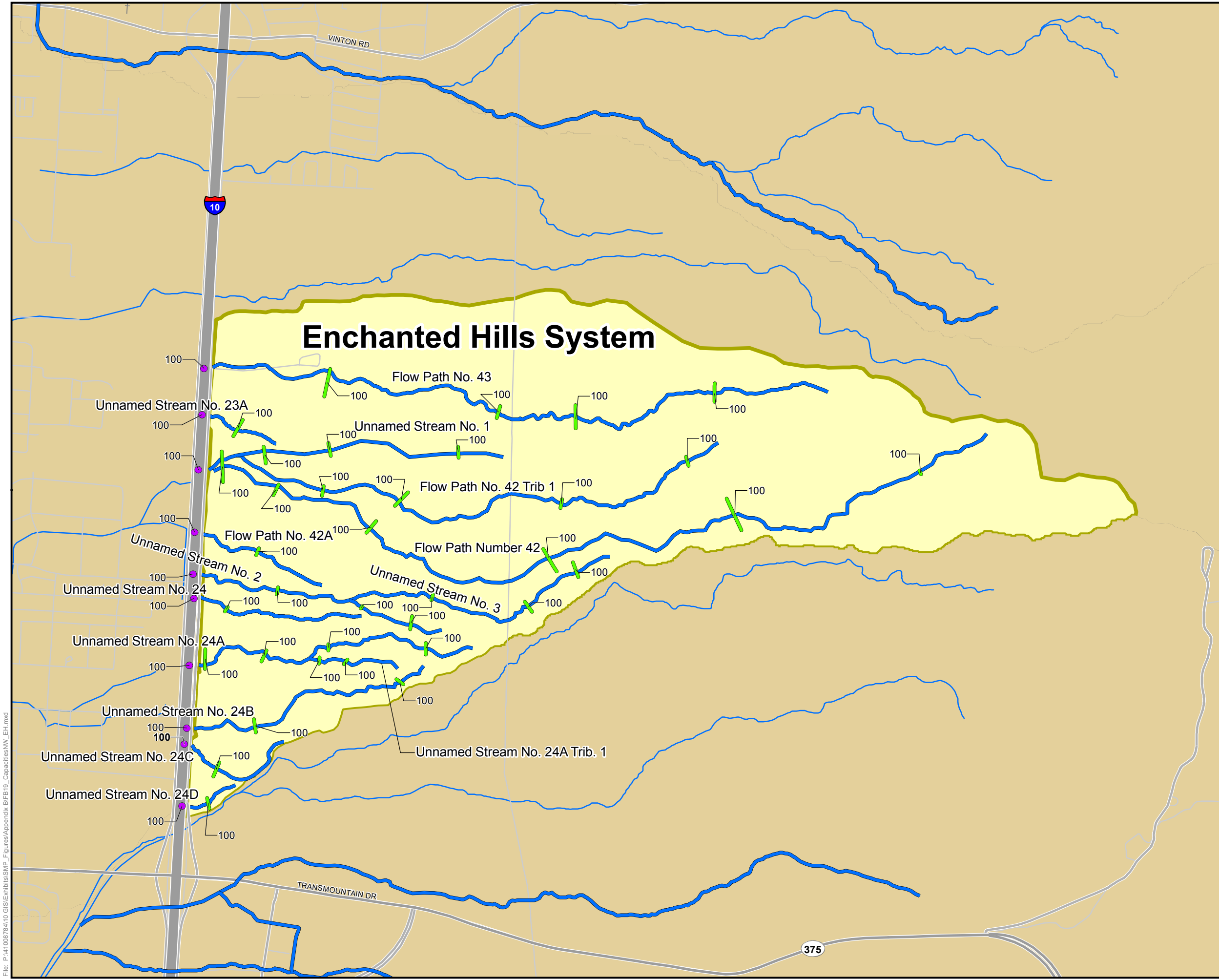


**Doniphan System
Channel and Crossing Return
Period Capacities
Northwest Region**

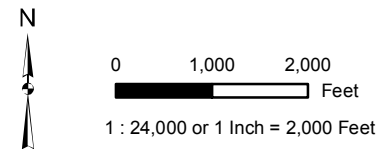
El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-18





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Enchanted Hills System
 - Other Drainage Systems



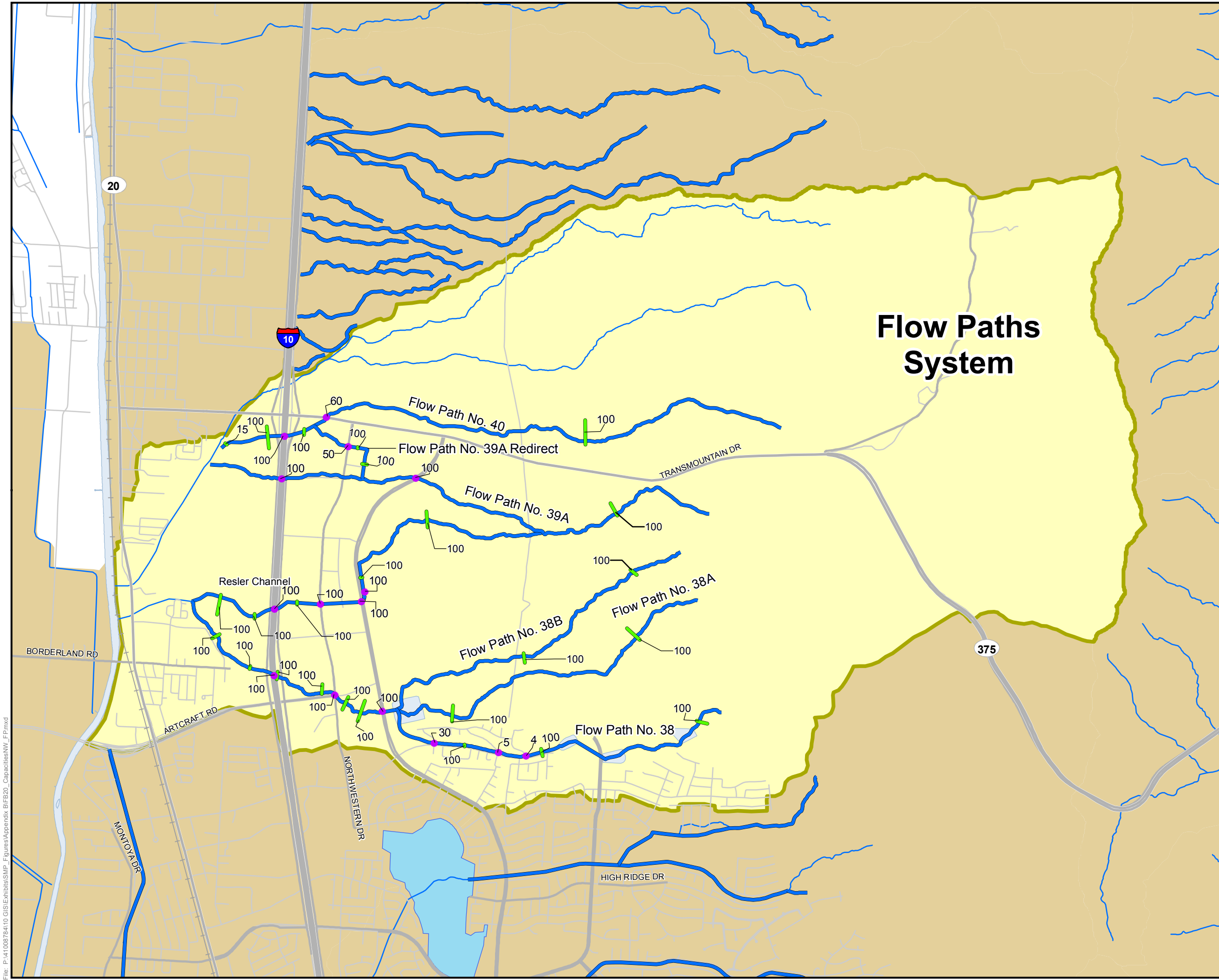
**Enchanted Hills System
Channel and Crossing Return
Period Capacities
Northwest Region**

El Paso Stormwater Master Plan

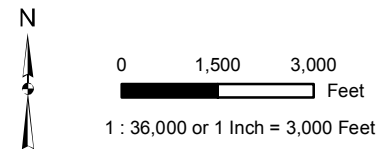
Date: 03/05/2009 Figure B-19



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- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - Flow Paths System
 - Other Drainage Systems



**Flow Paths System
Channel and Crossing Return
Period Capacities
Northwest Region**

El Paso Stormwater Master Plan

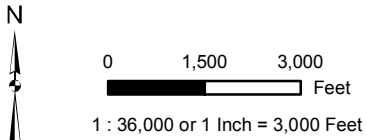
Date: 03/05/2009 Figure B-20



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- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Dam/Basin
 - Water Body
 - Keystone Dam System
 - Other Drainage Systems

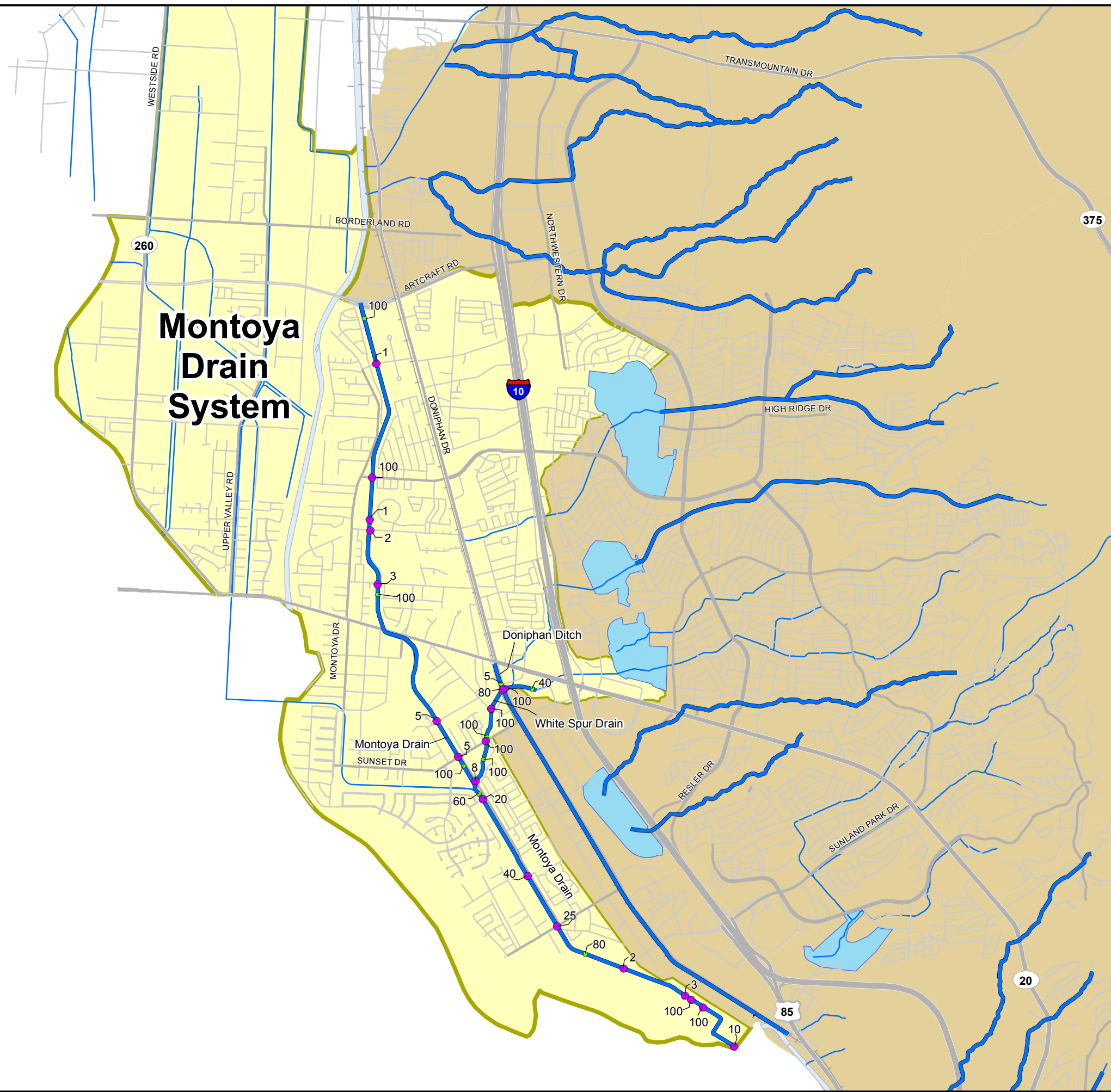


**Keystone Dam System
Channel and Crossing Return
Period Capacities
Northwest Region**

El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-21





Legend

- Crossing Capacity
- Channel Capacity
- Study Reach
- Streamline
- Major Road
- Minor Road
- Railroad
- Dam/Basin
- Water Body
- Montoya Drain System
- Other Drainage Systems



0 2,000 4,000 Feet
1 : 48,000 or 1 Inch = 4,000 Feet

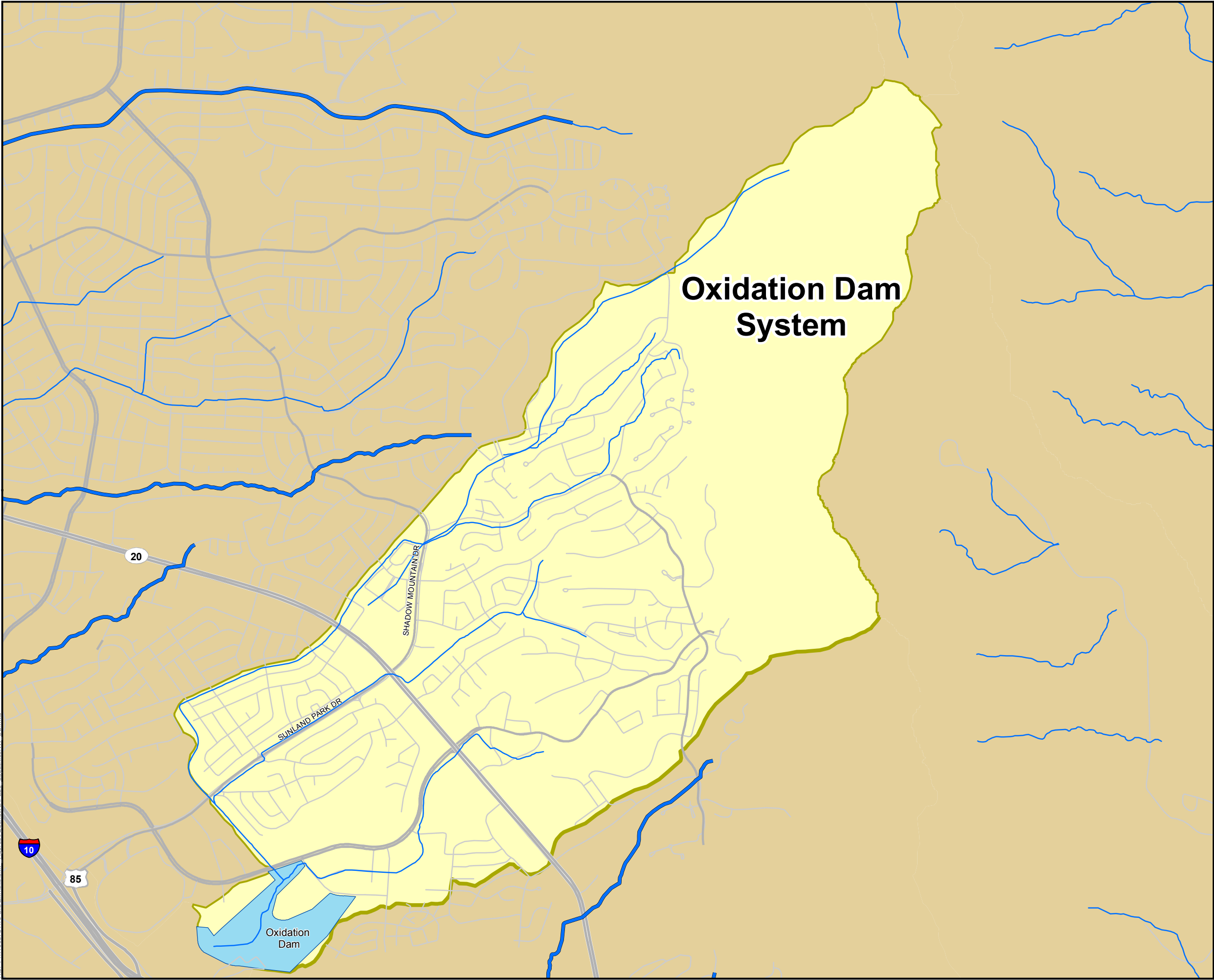
Montoya Drain System Channel and Crossing Return Period Capacities Northwest Region

El Paso Stormwater Master Plan

Date: 03/05/2009

Figure B-22





Legend

- Study Reach
- Streamline
- Major Road
- Minor Road
- Railroad
- Dam/Basin
- Oxidation Dam System
- Other Drainage Systems



0 1,000 2,000 Feet
1 : 24,000 or 1 Inch = 2,000 Feet

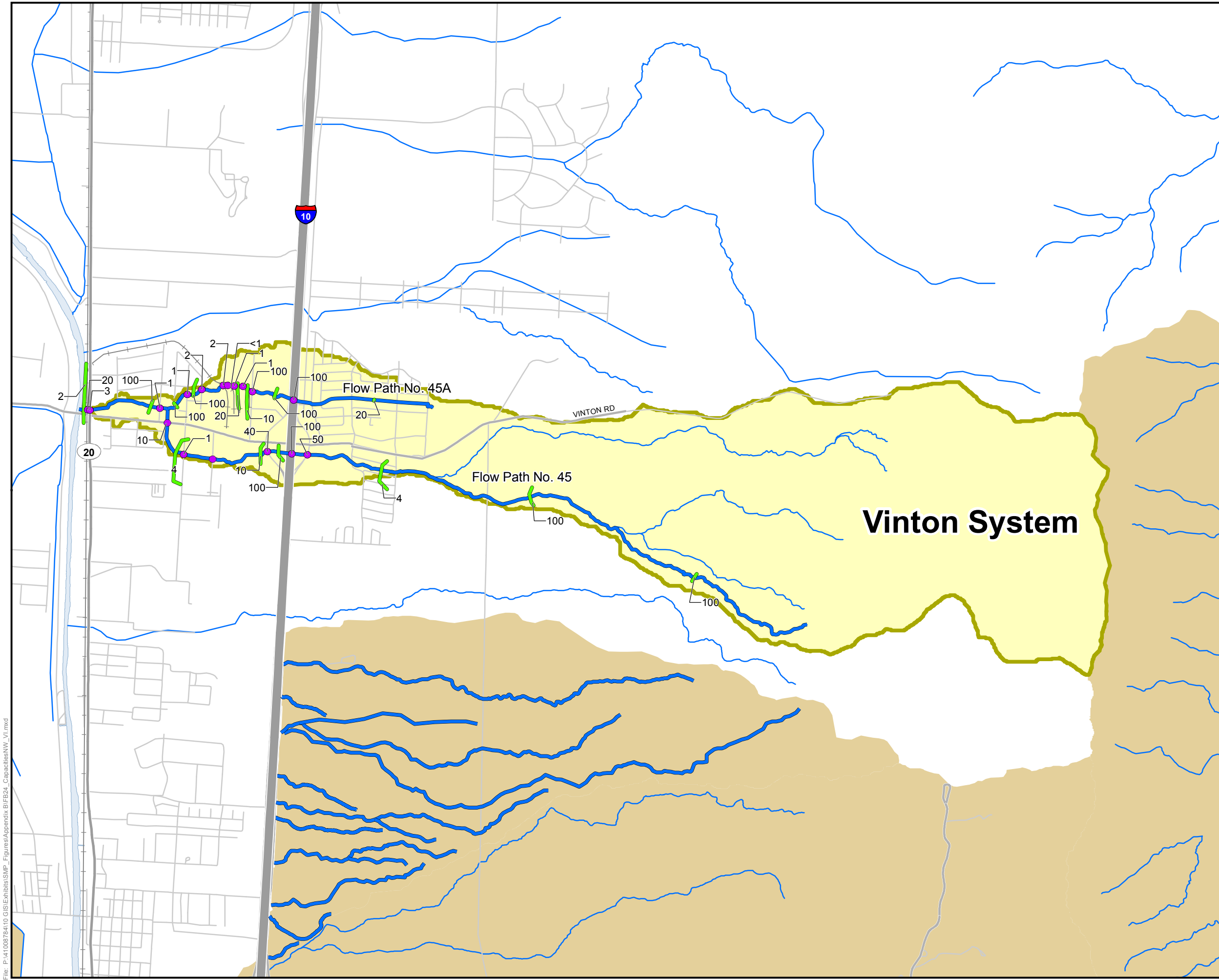
**Oxidation Dam System
Channel and Crossing Return
Period Capacities
Northwest Region**

El Paso Stormwater Master Plan

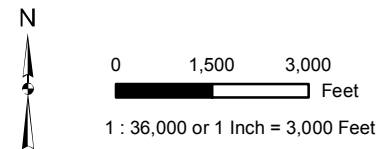
Date: 03/05/2009

Figure B-23





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Water Body
 - Vinton System
 - Other Drainage Systems

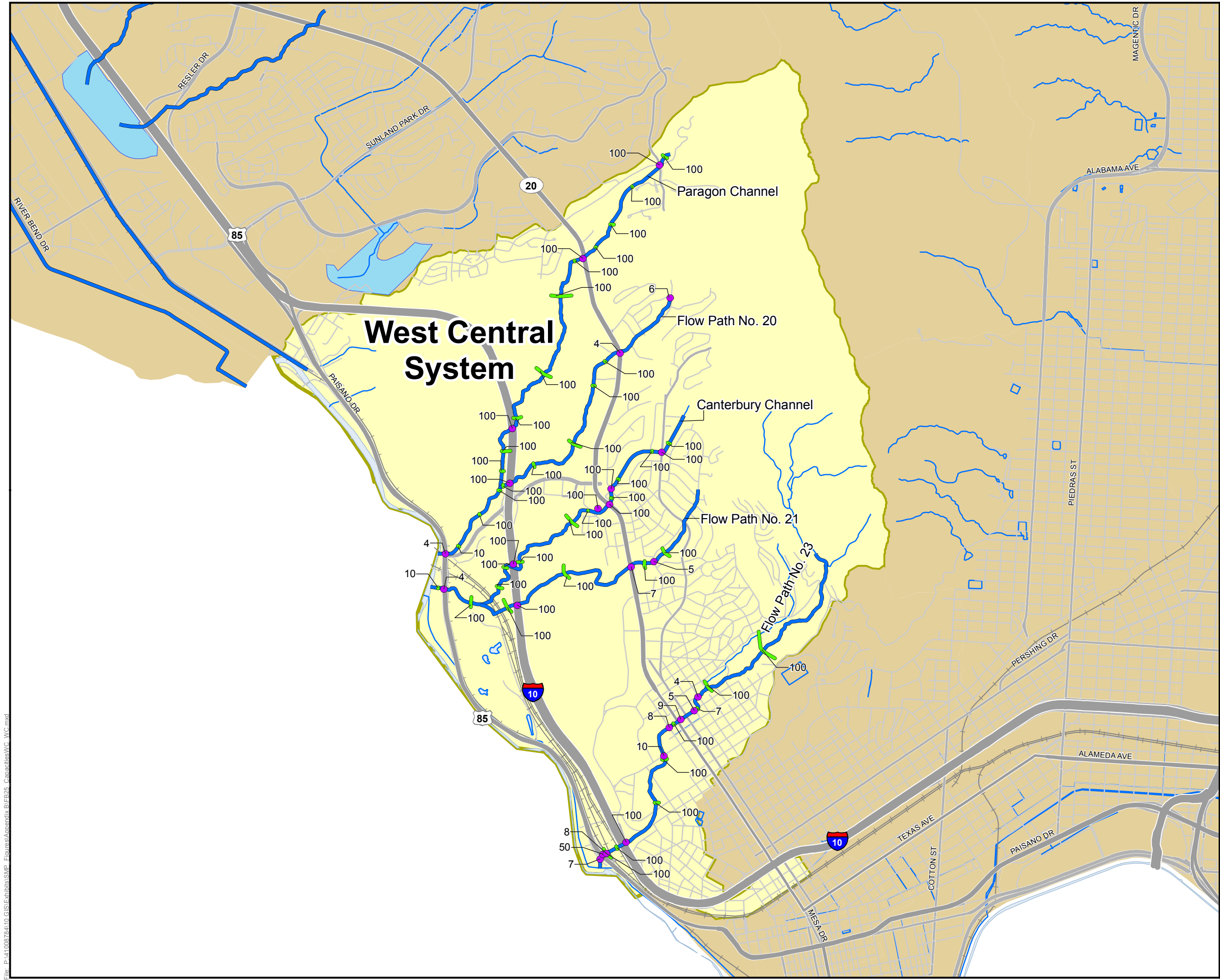


**Vinton System
Channel and Crossing Return
Period Capacities
Northwest Region**

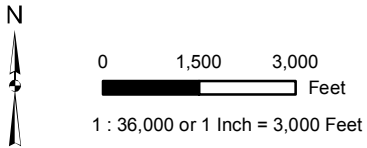
El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-24





- Legend**
- Crossing Capacity
 - Channel Capacity
 - Study Reach
 - Streamline
 - Major Road
 - Minor Road
 - Railroad
 - Dam/Basin
 - Water Body
 - West Central System
 - Other Drainage Systems



**West Central System
Channel and Crossing Return
Period Capacities
West Central Region**

El Paso Stormwater Master Plan

Date: 03/05/2009 Figure B-25

